

Municipality of Jasper

Tactical Level Asset Management Study (Phase 2) – Water Distribution, Wastewater Collection, and Roadways

- Interim

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Executive Summary

In December 2015, the Municipality of Jasper completed a Strategic Level Asset Management Report. This was a desktop analysis based on theoretical service life of the infrastructure assets. The strategic level analysis determined a long-range funding plan designed to provide the senior administration and Council guidance on capital renewal programming needs to attain infrastructure sustainability.

This report provides the next asset management phase, which is a Tactical Level assessment and analysis. It is more detailed involving a field level assessment of the infrastructure assets and a lifecycle modeling analysis designed to maximize the value for taxpayers while ensuring infrastructure sustainability over time. It was completed for the Roadways (i.e. streets and sidewalks), wastewater collection (i.e. sanitary and storm pipes, manholes, and catch basins), and water distribution (i.e. pipes and valves)

The results of the tactical level analysis determined a short-range detailed works program for every infrastructure segment and determined the long-range funding plan required to deliver infrastructure sustainability. The following table presents the summary of both the short-range and long-range funding needs.

| Asset Group | Tactical | | | | Strategic | Historic | Difference Sustainability to Budget Surplus (+); Deficit (-) (M\$/yr) |
|----------------|---|---|--|---|--|---|--|
| | Short-Term (5 Year) Needs Total (M\$) | Short-Term (5 Year) Needs Annual (M\$/yr) | Long-Term Sustainability Needs Annual (M\$/yr) | Reserve Fund Annual Accumulation (+) building; (-) drawing (M\$/yr) | Long-Term Sustainability Needs Projection (M\$/yr) | Current Budget Allocation (M\$/yr) | |
| Roadways | | | | | | | |
| - Streets | \$2.660 | \$0.532 | \$0.230 | | | | |
| - Sidewalks | <u>\$0.375</u> | <u>\$0.075</u> | <u>\$0.074</u> | | | | |
| Sub-Total | \$3.035 | \$0.607 | \$0.304 | -\$0.303 | \$1.220 | \$0.710 | \$0.406 |
| Wastewater | | | | | | | |
| - Collection | \$0.636 | \$0.127 | \$0.201 | | | | |
| - Treatment | <u>TBD</u> | <u>TBD</u> | <u>\$0.415</u> | | | | |
| Sub-Total | \$0.636 | \$0.127 | \$0.616 | \$0.489 | \$0.730 | \$0.130 | -\$0.486 |
| Water | | | | | | | |
| - Distribution | \$2.500 | \$0.500 | \$1.309 | | | | |
| - Treatment | <u>TBD</u> | <u>TBD</u> | <u>\$0.332</u> | | | | |
| Sub-Total | <u>\$2.500</u> | <u>\$0.500</u> | <u>\$1.641</u> | <u>\$1.141</u> | <u>\$0.940</u> | <u>\$0.310</u> | <u>-\$1.331</u> |
| Total | \$6.171 | \$1.234 | \$2.561 | \$1.327 | \$2.890 | \$1.150 | -\$1.411 |

The roadways streets have the most immediate need with a pavement open surface texture that can largely be addressed by a cost effective a mid-life preservation (i.e. micro-surfacing) treatment. The sanitary piping has some immediate needs that again could largely be addressed by cost effective mid-life (i.e. cured in place liner) treatment. However, the greater needs for both the sanitary and storm water collection system are expected in approximately 30-40 years respectively. The water distribution system is in relatively good condition given the age of the cast-iron piping network. However, the associated valves are running on a different lifecycle than the pipes they are connected too. The model is forecasting the risk of valve failure and allocating expenditures on associated emergency repair. As a result, valve repairs are the water main short-term expenditure needs. Water main replacement is expected to begin in approximately 14 years from now.

Overall, the Municipality should be spending \$1.234 Million/year for the short-term (i.e. 5 years) in capital renewal programming for the roadways, wastewater (storm and sanitary), and water infrastructure groups. However, to meet the long-term sustainability needs, the Municipality should be investing a total of \$2.561 Million/year. Such should be a consideration for external grant funding, tax and utility rates, and reserve funds.

Moving forward, the Municipality should be updating their infrastructure inventory GIS records. They should be developing maintenance management system to track associated works. They should be repeating the tactical level analysis every 2-4 years with greater focus on the water distribution system.

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Appendix A – Condition Assessment Criteria

Appendix B – Infrastructure Life-Cycle Optimization Modeling Output and Mapping

1. Introduction

In December 2015, the Strategic Asset Management Study was completed. It was a desktop study utilizing existing Tangible Capital Assets (TCA) and Geographic Information Systems (GIS) data, along with theoretical service life and replacement cost data, to develop a long-range funding plan for each asset group. The following table presents the Long-Range Funding Plan from the previous Strategic level report. Overall, it illustrates that the current budget allocation for infrastructure renewal is \$2.54 Million/yr. The projected budget allocation to address the infrastructure deficit ranges between \$5.85 Million/year and \$7.02 Million/year. The purpose of these findings at the strategic level was to prepare the Municipality of Jasper for capital renewal funding in their annual budget programming.

Table 1 – Long Range Funding Plan (Strategic Level Study) - 2016

| Asset Group | Replacement Cost (\$M) | Current Budget Allocation (\$M/yr) | Conventional | | Preservation Enhanced | |
|-------------------|------------------------|------------------------------------|---------------|--------------------------------|-----------------------|--------------------------------|
| | | | Backlog (\$M) | Capital Renewal Needs (\$M/yr) | Backlog (\$M) | Capital Renewal Needs (\$M/yr) |
| Roadways | 57.10 | 0.71 | 4.07 | 1.41 | 4.52 | 1.22 |
| Water | 45.70 | 0.31 | 6.25 | 0.94 | 6.25 | 0.94 |
| Sanitary | 41.60 | 0.11 | 1.29 | 0.89 | 3.90 | 0.52 |
| Storm Water | 31.00 | 0.02 | - | 0.82 | 0.01 | 0.21 |
| Land improvements | 4.20 | 0.11 | 0.97 | 0.20 | 0.97 | 0.20 |
| Buildings | 75.40 | 1.05 | 8.59 | 1.73 | 8.59 | 1.73 |
| Machinery | 5.00 | 0.18 | 1.61 | 0.38 | 1.61 | 0.38 |
| Vehicles | 8.90 | 0.05 | 3.35 | 0.65 | 3.35 | 0.65 |
| TOTAL | 268.90 | 2.54 | 26.13 | 7.02 | 29.20 | 5.85 |

Note * Shaded cells indicate preservation enhancements not currently explored in these asset groups

The purpose of this study is to provide more detailed analysis in to the state of the infrastructure including field level condition assessment. Then a lifecycle optimization analysis would be completed to determine a detailed program strategy for each infrastructure segment in the network. Even though the analysis works long-term (i.e. 20-40 year), the developed detailed program strategy is short-term (i.e. 5 year). After that, reassessment should be completed to consider the actual changes to the infrastructure condition state over time. This is often referred to as a tactical level analysis.

The first stage of the Tactical level analysis was completed for the Roadways (i.e. streets and sidewalks), Water (i.e. piping and valves), Sanitary (piping and manholes), and Storm Water (piping, manholes, and catch basins) asset groups. A different approach to treatment facilities, buildings, machinery, and vehicles will be completed later in a separate analysis.

2. Condition Assessment and Lifecycle Analysis Process

Condition rating criteria was developed for each infrastructure (i.e. asset) group. The criteria were based in part on other industry criteria practices. However, it was tailored to the Municipality of Jasper. The condition rating criteria defined for the Municipality of Jasper's infrastructure groups is contained in Appendix A. The following table summarizes the condition types assessed for each infrastructure group.

Table 2 – Condition Types

| <u>Infrastructure Group</u> | <u>Condition Type</u> |
|-------------------------------|---|
| Roadways (Streets) | Rutting |
| | Lineal Cracking |
| | Fatigue Cracking (reflection of structural failure) |
| | Surface Condition (i.e. ravelling and open surface texture) |
| | Grade (curb and gutter loss due to historic overlays) |
| Sidewalks | Cracking |
| | Spalling (i.e. open surface texture) |
| Water Mains | Structural Pipes (remaining wall thickness) |
| | Structural Valves |
| | Capacity (Noted for 4" pipe mains) |
| Sanitary Mains | Structural (NASSCO PACP Grade) |
| | O & M (NASSCO PACP Grade) |
| | Capacity (As per CCTV assessment) |
| Sanitary Manholes | Structural (NASSCO PACP Grade) |
| | O & M (NASSCO PACP Grade) |
| | Capacity (As per CCTV assessment) |
| Storm Mains | Structural (NASSCO PACP Grade) |
| | O & M (NASSCO PACP Grade) |
| | Capacity (As per CCTV assessment) |
| Storm Manholes & Catch Basins | Structural (NASSCO PACP Grade) |
| | O & M (NASSCO PACP Grade) |
| | Capacity (As per CCTV assessment) |

The fundamental component of condition assessment criteria for each infrastructure group is severity and extent. Severity is a defined measure of the level of deterioration (i.e. minor, moderate, major, and severe). The extent is the proportion of the infrastructure segment surface area within each of the defined severity levels. As example, the following table illustrates the water main structural assessment based on the measured remaining wall thickness.

Table 3 – Water Main Structural Assessment Criteria

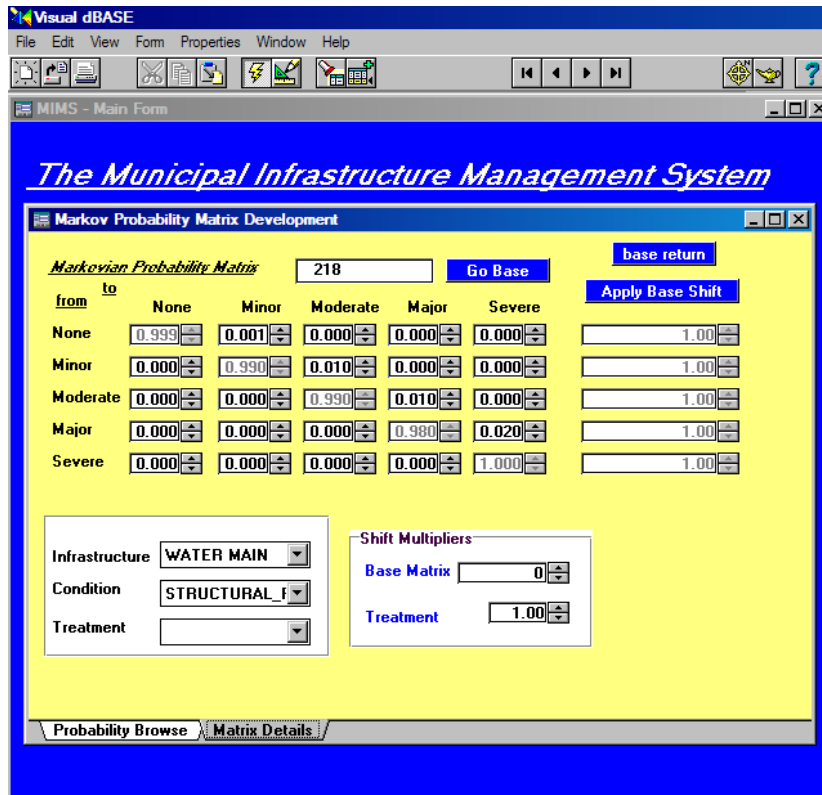
| Severity Level | Criteria | Extent (%) |
|----------------|------------------------------------|--------------------|
| None | Remaining Wall Thickness = 80%-90% | Contractor Reports |
| Minor | Remaining Wall Thickness = 50%-80% | “ |
| Moderate | Remaining Wall Thickness = 20%-50% | “ |
| Major | Remaining Wall Thickness = 5%-20% | “ |
| Severe | Remaining Wall Thickness = 0%-5% | “ |

Deterioration probability matrices are developed for each condition type. Historic data measurements were in part used to develop the deterioration rates and probability of moving from one severity level to another. The following figure illustrates the framework of deterioration probabilities. The following figure illustrates the deterioration probability matrix for the Jasper Water Main Structural (i.e. remaining wall thickness) condition type.

Table 4 – Deterioration Probability Matrix Framework

| | | To: | | | | | |
|-------|--------|-----------------|-----------------|-----------------|-----------------|-----------------|-------|
| | | None | Minor | Mod. | Major | Severe | Total |
| From: | None | P ₁₁ | P ₁₂ | P ₁₃ | P ₁₄ | P ₁₅ | 1.0 |
| | Minor | P ₂₁ | P ₂₂ | P ₂₃ | P ₂₄ | P ₂₅ | 1.0 |
| | Mod. | P ₃₁ | P ₃₂ | P ₃₃ | P ₃₄ | P ₃₅ | 1.0 |
| | Major | P ₄₁ | P ₄₂ | P ₄₃ | P ₄₄ | P ₄₅ | 1.0 |
| | Severe | P ₅₁ | P ₅₂ | P ₅₃ | P ₅₄ | P ₅₅ | 1.0 |

Figure 1 – Deterioration Probability Matrix – Jasper Water Main Structural Condition



Markovian Probabilistic Modelling principles use the measured severity and extent data with deterioration probabilities to forecast condition deterioration over time (i.e. several years). The following figure illustrates the Markovian deterioration calculation process.

Figure 2 – Markovian Simulated Condition Extent Calculation

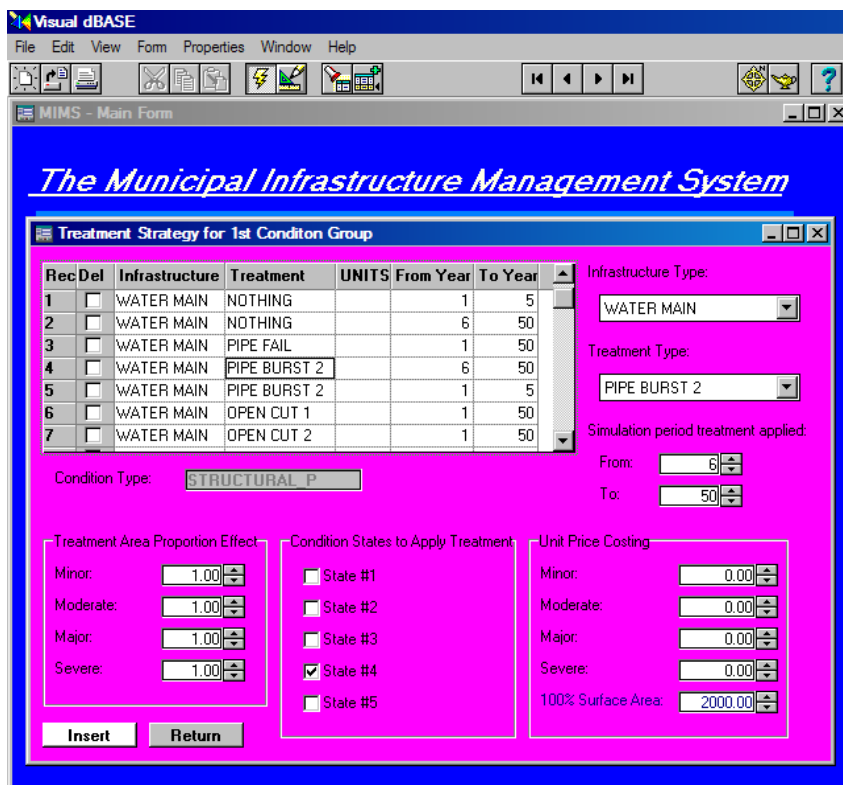
| Extent Levels within each Severity Rating | | | | | | |
|---|--|--|--|--|--|----------------|
| Year | None | Minor | Moderate | Major | Severe | Index |
| Y ₀ | E ₀₁ | E ₀₂ | E ₀₃ | E ₀₄ | E ₀₅ | I ₀ |
| Y ₁ | E ₀₁ *P ₁₁ + E ₀₂ *P ₂₁ + E ₀₃ *P ₃₁ + E ₀₄ *P ₄₁ + E ₀₅ *P ₅₁ | E ₀₁ *P ₁₂ + E ₀₂ *P ₂₂ + E ₀₃ *P ₃₂ + E ₀₄ *P ₄₂ + E ₀₅ *P ₅₂ | E ₀₁ *P ₁₃ + E ₀₂ *P ₂₃ + E ₀₃ *P ₃₃ + E ₀₄ *P ₄₃ + E ₀₅ *P ₅₃ | E ₀₁ *P ₁₄ + E ₀₂ *P ₂₄ + E ₀₃ *P ₃₄ + E ₀₄ *P ₄₄ + E ₀₅ *P ₅₄ | E ₀₁ *P ₁₅ + E ₀₂ *P ₂₅ + E ₀₃ *P ₃₅ + E ₀₄ *P ₄₅ + E ₀₅ *P ₅₅ | I ₁ |

During the lifecycle simulation period, the modelled severity-extent forecasts are indexed to five condition states:

- 1 – very good
- 2 – good
- 3 – fair
- 4 – poor
- 5 – very poor

This is used to trigger treatment options at various stages in the infrastructure lifecycle. The following figure illustrates the water main “Pipe Busting” treatment option.

Figure 3 – Jasper Water Main – Pipe Bursting Treatment Option



In this example, the pipe bursting option is triggered in condition state “4”. Condition state “5” would be failure of the pipe. Condition state 4 triggers remediation at a point in the lifecycle just prior to high risk of failure.

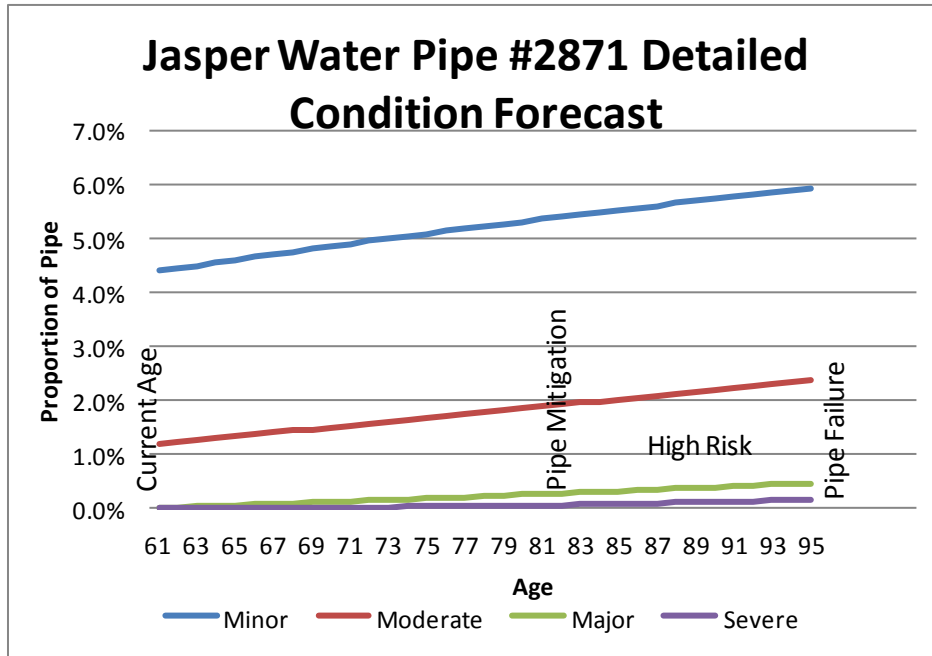
During the condition assessment process, pipe #2871 was assessed using electromagnetic technology. The assessment computed the structural rating based on severity-extent measurements of the pitting (i.e. remaining wall thickness). During the field testing, a portion of the pipe was extracted for physical verification of the electronic data measurements. The following figure illustrates a photo of the pipe sample noting the pitting within the pipe.

Figure 4 – Jasper Water Main #2871



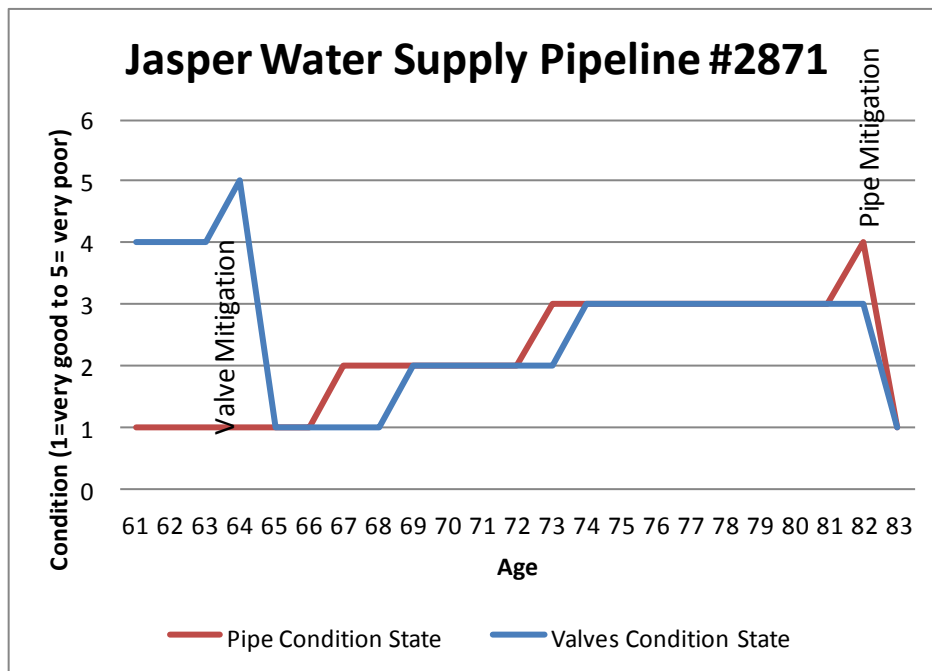
Pipe segment #2871 is a 61 year old cast-iron 150 mm (6”) water main. It is located in Patricia Circle. Overall, the pipe is in reasonably good condition. It has approximately 5-6% of the surface area pitted. The majority is in the minor severity grouping (i.e. 50-80% remaining wall thickness). There is no major or sever pitting. Starting from the measured condition state in year 61, the following figure shows the Markovian performance prediction for this pipe segment. The pipe may likely reach the end of its service life in 30-35 years from today. However, it would be prudent to address remediation ahead of the high risk period (i.e. 20-25 years from today).

Figure 5 – Severity Level Performance Prediction - Water Main #2871



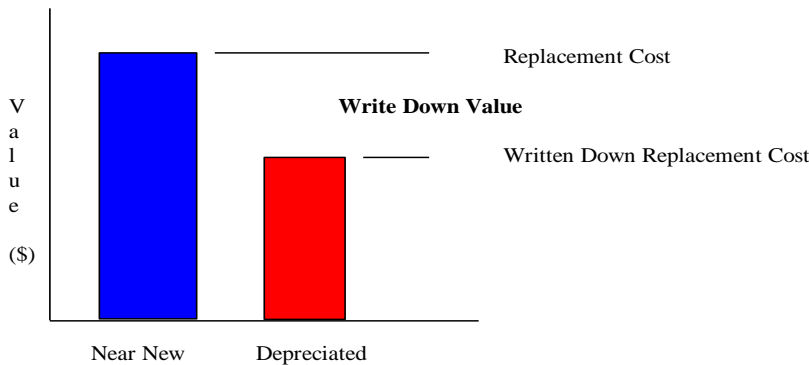
The following figure illustrates the condition state modelled over time with the treatment options applied. In this example, the graph shows two condition parameters; one for the pipe and the other for the valves. Today, the pipe is still in a very good (state 1) condition state. The associated valves along the line are projecting near the end of its service life and showing they will be replaced soon, or have been recently replaced. The pipe is showing reaching a poor (state 4) condition state in approximately 20 years from now. The modeling treatment strategy forecasted a “Pipe Bursting” treatment at this time, which reset the condition state of the pipe and associated valves to a very good (state 1) condition state.

Figure 6 – Condition State Performance Prediction - Water Main #2871



The current and modelled infrastructure condition is reported by the physical condition state (i.e. 1-5) for each of the condition types (i.e. fatigue cracking, etc.). The condition is also measured as monetary performance. This is the asset write-down-value (WDV). There is a relationship between condition state and WDV. The more deteriorated the infrastructure segment, the more depreciated the asset is and the higher the resulting WDV. This provides a dollar to dollar comparison between expenders or investment in the infrastructure asset to what the asset is worth. It enables the Municipality to determine if it is getting a positive Return on Infrastructure Investment (ROI).

Figure 7 – Write-Down-Value Illustration



The lifecycle modeling analysis completes the above example analysis for all the other infrastructure segments and for each infrastructure group (i.e. roadways, sidewalks, water mains, sanitary pipes, sanitary manholes, and storm manholes & catch basins). The condition assessment for the roadways and sidewalks was completed for each infrastructure in the infrastructure inventory. In some cases, we added segments that were missing from the inventory (i.e. GIS) system. The other infrastructure groups included only a sample of the network that was assessed. Then based on the pipe age and material type, a condition assessment was correlated for the remaining pipes that were not assessed. Our compiled data notes which pipes were physically assessed and which were rated based on a correlation.

Even though the modeling process is looking in the long-range horizon (i.e. 20-40 years), the focus for maintenance and capital programming purposes will be the short-range horizon (i.e. 5 year). It would be expected condition assessments be repeated in 2-4 year increments.

The same analysis process is applied to the sewer mains, manholes & catch basins, roadways (streets) and sidewalks.

3. Current State of the Infrastructure

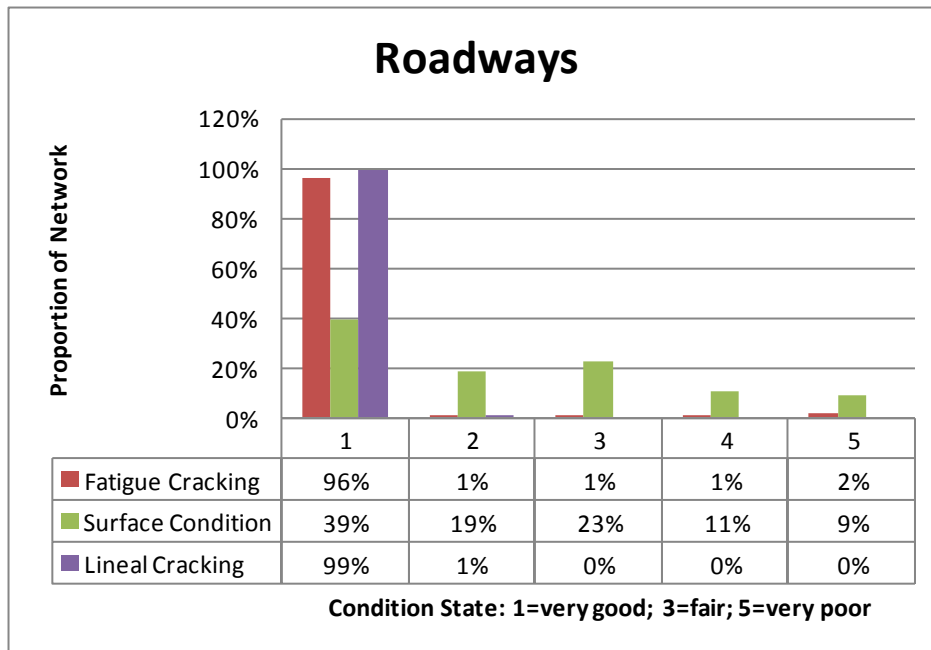
As a precursor to the tactical level analysis, this section presents the current state of the infrastructure. This is as per the field level condition assessments completed in the summer of 2016. This provides some understanding taken in the infrastructure renewal strategy.

3.1 Roadways (Streets - Pavement)

The following figure illustrates the current condition state for the roadways network. It illustrates the proportion of the network in each of the five condition states (i.e. 1=very good; 3=fair; 5=very poor). This is illustrated for the fatigue cracking, surface condition, and lineal cracking condition types. Not shown on the graph is rutting and grade. These were also assessed in the field but with negligible issues. As such, rutting and grade are noted in a very good condition state.

Overall, the roadways are in fairly good condition. They are structurally sound with no indication of subsurface failure. The primary concern is poor to very poor surface condition on approximately 20 percent of the roadways. The surface is beginning to ravel. This is a loss of binder around the aggregate and the roadways are taking on an exposed aggregate appearance. In some cases, this has progressed to a loss of aggregate and eventual potholes. Timely remediation could seal the surfaces to a near new condition state. However, if deterioration progresses, then full pavement rehabilitation will be required at five times the cost of preventative maintenance such as a micro-surface. Roadways are in a timely sensitive stage in its lifecycle. Deferral of works could significantly increase the cost to renew this infrastructure group.

Figure 8 – Roadways State of the Infrastructure

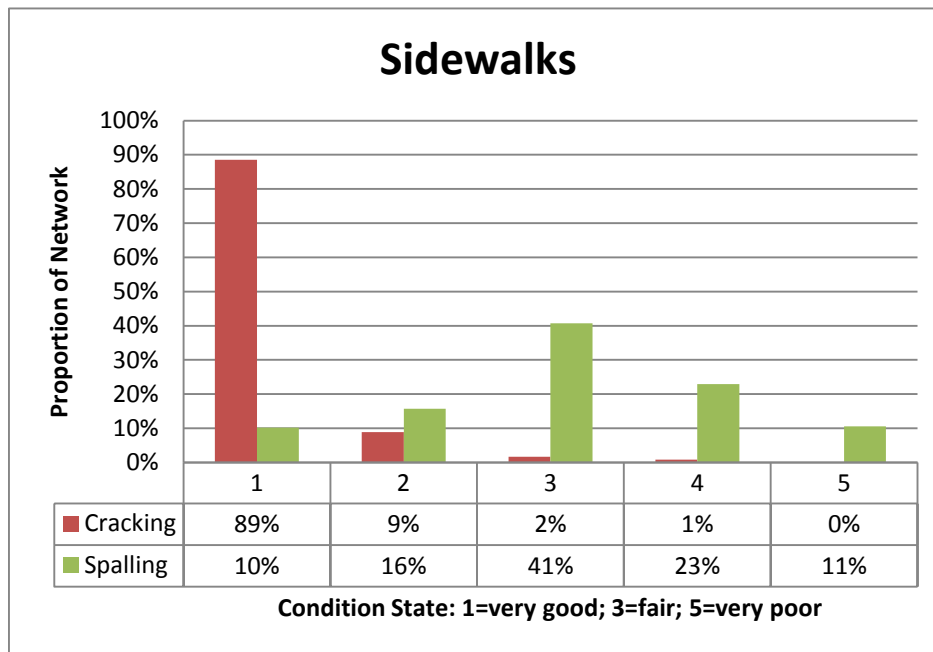


3.2 Sidewalks (Concrete)

The following figure illustrates the current condition state for the sidewalks network. It illustrates the proportion of the network in each of the five condition states (i.e. 1=very good; 3=fair; 5=very poor). This is illustrated for the cracking and spalling condition types.

Overall, the sidewalks are in fairly good condition. They are structurally sound with no indication of subsurface failure. This is shown by very little cracking and heaving. The primary concern is the poor to very poor spalling on approximately 34 percent of the sidewalks. This is often a result of the aggregate quality and/or salts used for roadway de-icing. On inspection, it was observed that some of the newer sidewalks were spalling quicker than the older sidewalks. This is an indication that quality control may have changed over recent years.

Figure 9 – Sidewalks State of the Infrastructure



3.3 Wastewater Collection (Sanitary, Storm, Manholes and Catch Basins)

The wastewater collection infrastructure group was assessed as per the following sub-categories:

- Sanitary Mains
- Storm Mains
- Sanitary Manholes
- Storm Manholes and Catch Basins

The following figures present the Sanitary Mains and Storm Mains state of the infrastructure. The sanitary mains network appears to be in a good condition state. Approximately 2 percent of the network may have a structural concern requiring some form of attention. The more significant issues are operations and maintenance (i.e. O&M) relating to build up of scale along the pipe wall and roots infiltrating through pipe joints. Alternatively, the storm mains network appears to be in very good condition from both the structural and O&M perspective. This is likely related to the nature of the sanitary effluent, due to its corrosiveness and nutrient content.

Figure 10 – Sanitary Mains State of the Infrastructure

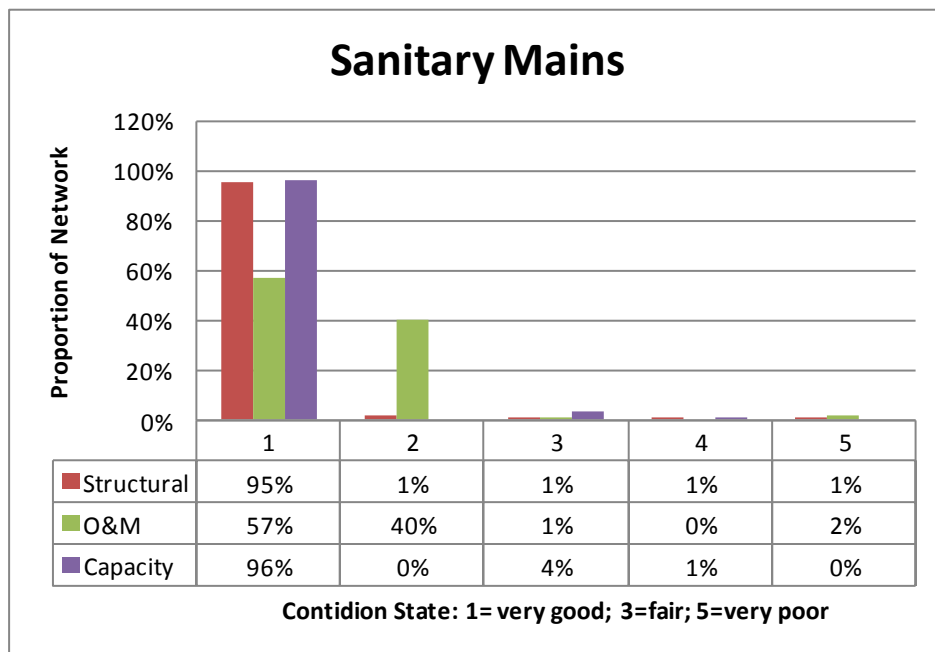
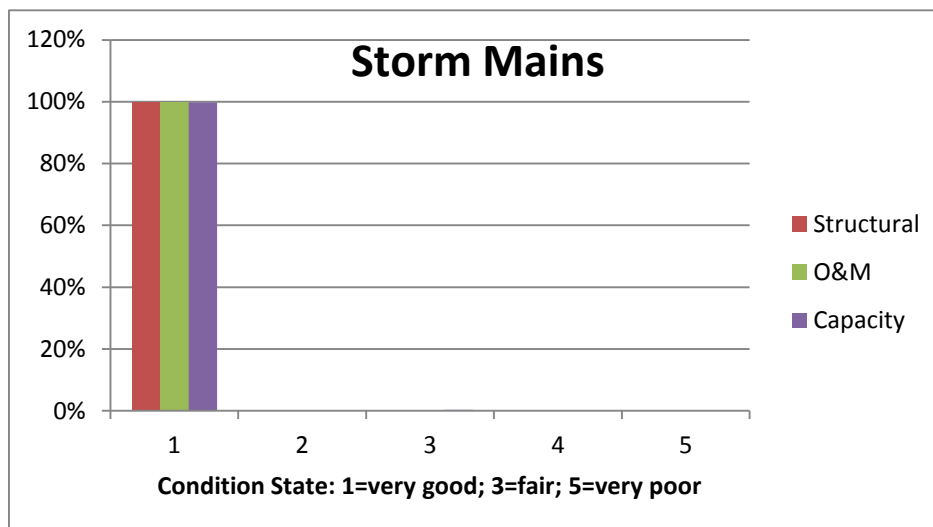


Figure 11 – Storm Mains State of the Infrastructure



The following figures presents the Sanitary Manholes and Storm Mains & Catch Basins state of the infrastructure. Both groupings appear to be in a very good condition state.

Figure 12 – Sanitary Manholes State of the Infrastructure

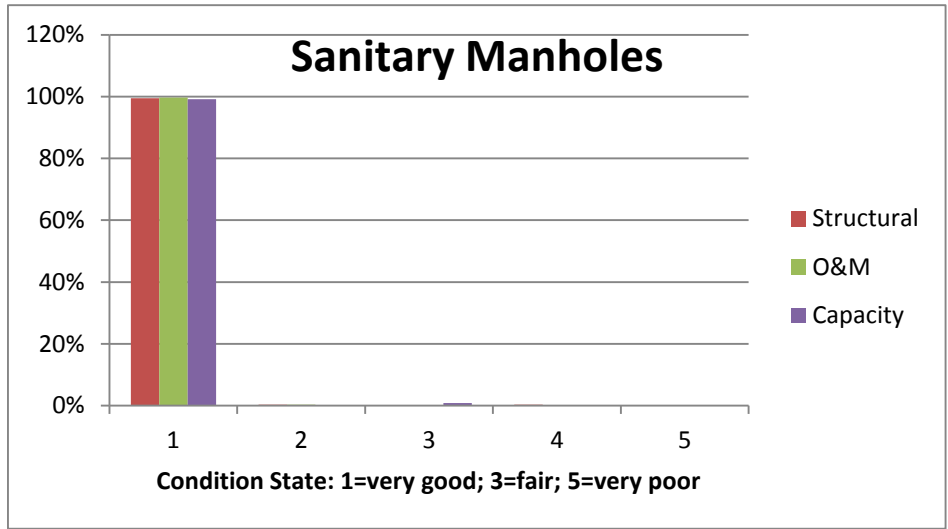
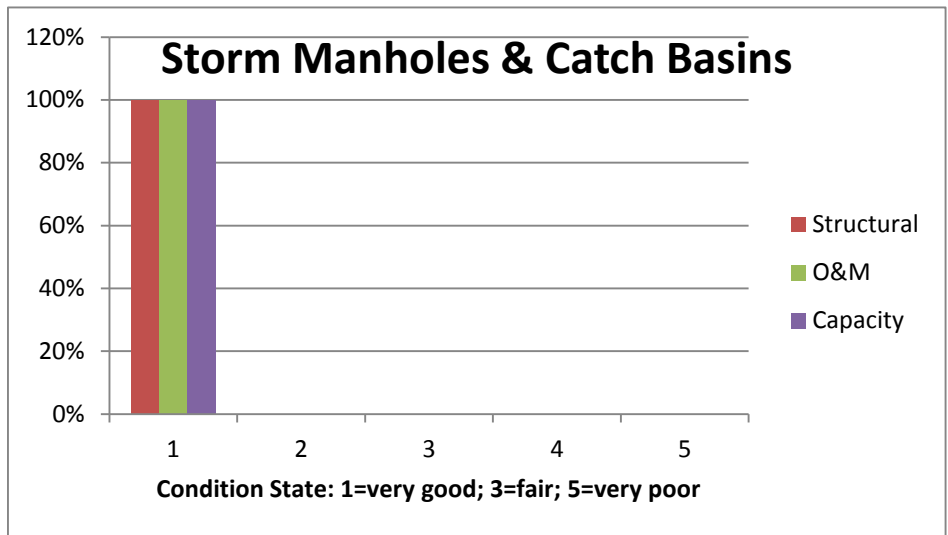


Figure 13 – Storm Manholes & Catch Basins State of the Infrastructure



3.4 Water Distribution (Pipes and Valves)

The following figure presents the Water Mains state of the infrastructure. Water mains were the most challenging infrastructure group in acquiring condition data. The municipality had very little information on the pipe break history. Most occurrences were noted around valve failures, not necessarily the pipe. Very good pipe condition data was however attained for one 61 year old cast iron pipe segment (#2871) in Patricia Circle. This was the correlation sample for the rest of the network. At this time it was noted some recent valve/hydrant replacements on Patricia Circle. A valve assembly was also being replaced on pipe segment #2871 concurrent with the data gathering. This provided some indication that the service life of the water main valves was shorter than the pipe segments. This was a factor in the tactical level analysis moving forward.

Figure 14 – Water Mains State of the Infrastructure

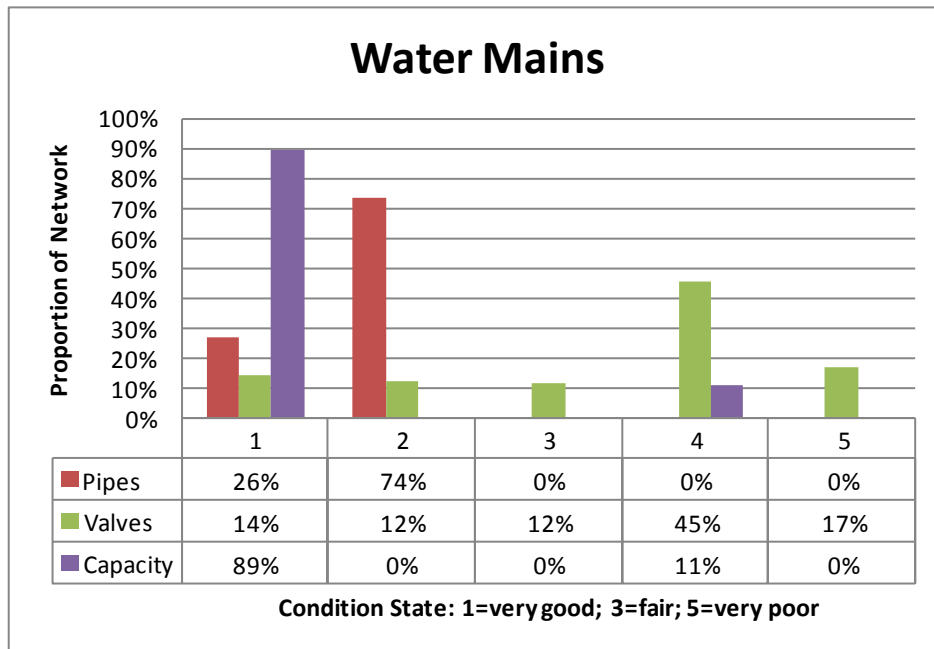


Figure 15 – Patricia Circle Water Main Pipe #2871 Valve Assembly Replacement (Year 2016)



4. Tactical Asset Management

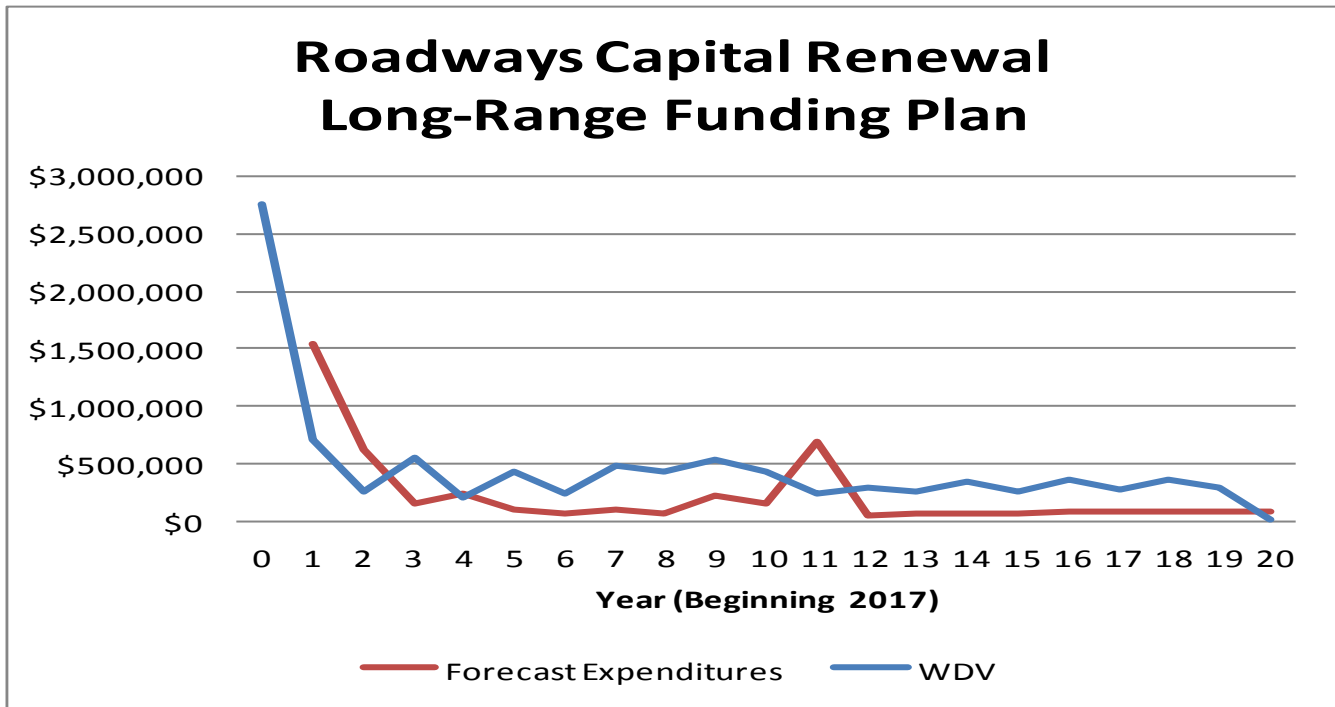
The tactical level analysis begins with the current state of the infrastructure and models the deterioration over time. During the optimization modeling analysis, alternative maintenance and renewal treatments are triggered as appropriate to do so within the infrastructure lifecycle. The analysis is completed for every segment within each infrastructure group.

The objective is to develop a plan that will proactively sustain the infrastructure over its lifecycle while minimizing costs.

4.1 Roadways (Streets)

As presented earlier in the State of the Infrastructure section, open surface texture is the primary condition distress. As a result, the optimal scenario is triggering an aggressive micro-surfacing program in the short-term (5-year) horizon; along with some resurfacing (i.e. repaving) where the level of deterioration has accelerated. The following figure summarizes the projected spending over the 20-year horizon.

Figure 16 – Roadways Long-Range Funding Projection



The following table measures the effectiveness of the proposed spending in the short-term (i.e. 5-year) and long-term (i.e. 20-year) horizons. The short term has more capital renewal spending due to the backlog of mitigating the pavement’s open-surface texture. In the short-term the Municipality should be in a position to be investing in capital renewal estimated at approximately \$532,000/year. With this spending is a significant improvement in the roadways condition state estimated at 9-17%/year in the short term and 3-5%/year in the long term. This is indicative of the ROI, which demonstrates a significantly greater improvement in the asset value given the maintenance and renewal investment.

Table 5 – Roadways Performance Summary

| Roadways | Optimization 5-year | Optimization 20-year |
|-----------------------------------|--------------------------------|---------------------------------|
| Program Expenditures | | |
| - Total Period Expenditure (M\$) | \$2.662 | \$4.607 |
| - Annualized Expenditure (M\$/yr) | \$0.532 | \$0.230 |
| Monetary Performance | | |
| - WDV (initial) | \$2.759 | \$2.759 |
| - WDV (End) | <u>\$0.427</u> | <u>\$0.012</u> |
| Improved Perf (+) | \$2.332 | \$2.747 |
| Annual Perf Change (M\$/yr) | \$0.466 | \$0.137 |
| | 17% | 5% |
| Condition State/Index | | |
| - Index (initial) | 14 | 14 |
| - Index (End) | <u>8</u> | <u>7</u> |
| Improved Condition (+) | 6 | 7 |
| Annual Condition Change (/yr) | 1.2 | 0.4 |
| Annual Condition Change (%/yr) | 9% | 3% |
| ROI - Annualized (%/yr) | 188% | 160% |
| - (100% = Stabilization Level) | | |

Within the five-year horizon, the following table summarizes the annualized cost broken out by maintenance (i.e. patching and crack filling), micro-surfacing, and resurfacing. The initial five years are relatively aggressive at a projected spending of \$532,000/yr.

Table 6 – Roadways Five-Year (2017-2021) Maintenance and Renewal Summary

| <u>Treatment Activity</u> | <u>Length (m)</u> | <u>Cost (\$/yr)</u> |
|-----------------------------------|--------------------------|----------------------------|
| Maintenance | | \$30,000 |
| Micro-Surfacing (i.e. Micro-Seal) | 16,009 | \$340,000 |
| Resurfacing (i.e. Repaving) | 1,446 | <u>\$162,000</u> |
| Average Annual Cost | | \$532,000 |
| | | |
| Total Cost Over 5-Years | | \$2,660,000 |

The micro-surfacing (sometimes called micro-sealing) treatment is new to the municipality of Jasper. It is a unique blend of manufactured aggregate fines, emulsion, and portland cement applied in a thin 7 mm lift; much thinner than a typical 50 mm asphalt concrete resurfacing lift. The final product looks similar to a new asphalt concrete surface. It is very effective at mitigating the open surface texture. It has some minor effect on the fatigue cracking. However, if the roadway is too deteriorated, then a conventional resurfacing operation is required. However, the cost of resurfacing is approximately four-times the cost of micro-surfacing. The following figure illustrates a typical micro-surfacing operation.

Figure 17 – Micro-Surfacing



The following tables list the specific road segments targeted for Micro-Surfacing and Resurfacing in the five-year horizon. Included is the condition state for the existing and modelled condition for each of the major distresses. The condition rating score is as follows:

- 1 – very good
- 2 – good
- 3 – fair
- 4 – poor
- 5 – very poor

As observed in the tables, below, micro-surfacing is predominately being generated for fair to poor surface condition. The resurfacing is primarily being generated for poor to very poor fatigue cracking and very poor surface condition.

Table 7– Five-Year (2017-2021) Roadways Micro-Surfacing Segment Listing

| <u>SEGMENT ID</u> | <u>LOCATION</u> | <u>AGE</u> | <u>LENGTH</u> | <u>YEAR</u> | <u>TREATMENT</u> | <u>COST</u> | <u>Fatigue Cracking</u> | <u>Surface Condition</u> | <u>Lineal Cracking</u> |
|-------------------|--------------------------------|------------|---------------|-------------|------------------|-------------|-------------------------|--------------------------|------------------------|
| 30EB | Connaught Drive North | 1994 | 204 | 3 | MICRO SEAL | \$ 32,795 | 1 | 3 | 2 |
| 30EB | Connaught Drive North | 1994 | 204 | 3 | MICRO SEAL | \$ 32,795 | 1 | 3 | 2 |
| 30EE | Connaught Drive North of Sawri | 1994 | 546 | 3 | MICRO SEAL | \$ 43,888 | 1 | 3 | 2 |
| 3110 | Geikie Street 100 Block | 2010 | 204 | 2 | MICRO SEAL | \$ 15,684 | 1 | 3 | 1 |
| 3111 | Patricia Street 100 Block | 1991 | 202 | 1 | MICRO SEAL | \$ 14,544 | 1 | 3 | 1 |
| 3114 | Aspen Ave | 1991 | 44 | 1 | MICRO SEAL | \$ 3,168 | 1 | 3 | 1 |
| 3115 | Aspen Ave | 1991 | 49 | 1 | MICRO SEAL | \$ 3,528 | 1 | 3 | 1 |
| 3117 | Geikie Street 200 Block North | 2010 | 62 | 2 | MICRO SEAL | \$ 4,780 | 1 | 3 | 1 |
| 3118 | Geikie Street 200 Block middle | 2010 | 67 | 1 | MICRO SEAL | \$ 4,824 | 1 | 3 | 1 |
| 3119 | Geikie Street 200 Block South | 2010 | 45 | 1 | MICRO SEAL | \$ 3,240 | 1 | 3 | 1 |
| 311A | Patricia Street 200 Block | 1991 | 175 | 1 | MICRO SEAL | \$ 12,667 | 1 | 3 | 1 |
| 311E | Balsam Ave | 1975 | 43 | 5 | MICRO SEAL | \$ 3,713 | 1 | 3 | 1 |
| 311F | Balsam Ave | 2000 | 44 | 5 | MICRO SEAL | \$ 3,800 | 1 | 3 | 1 |
| 3121 | Geikie Street 300 Block North | 2010 | 46 | 1 | MICRO SEAL | \$ 3,312 | 1 | 3 | 1 |
| 3122 | Geikie Street 300 Block South | 2010 | 117 | 1 | MICRO SEAL | \$ 8,424 | 1 | 3 | 1 |
| 3128 | Patricia Street 300 Block | 1991 | 162 | 1 | MICRO SEAL | \$ 14,774 | 1 | 3 | 1 |
| 3129 | Geikie Street 400 Block | 1991 | 222 | 1 | MICRO SEAL | \$ 26,725 | 1 | 4 | 1 |
| 3130 | Elm Ave | 1991 | 44 | 1 | MICRO SEAL | \$ 3,185 | 1 | 3 | 1 |
| 3131 | Elm Ave | 1991 | 44 | 1 | MICRO SEAL | \$ 3,185 | 1 | 3 | 1 |
| 3132 | Patricia Street 400 Block | 1991 | 222 | 1 | MICRO SEAL | \$ 37,339 | 1 | 5 | 1 |
| 313C | Bonhome Street 200 Block middl | 1970 | 131 | 1 | MICRO SEAL | \$ 9,457 | 1 | 3 | 1 |
| 313D | Aspen Ave | 1991 | 21 | 3 | MICRO SEAL | \$ 1,684 | 1 | 3 | 1 |
| 313E | Aspen Ave | 1991 | 68 | 3 | MICRO SEAL | \$ 5,453 | 1 | 3 | 1 |
| 313F | Aspen Ave | 1991 | 63 | 3 | MICRO SEAL | \$ 5,052 | 1 | 3 | 1 |
| 3140 | Bonhomme St inbetween Aspen Cr | 1970 | 86 | 1 | MICRO SEAL | \$ 6,225 | 1 | 3 | 1 |
| 3146 | Bonhomme St 200 Block South | 1977 | 269 | 1 | MICRO SEAL | \$ 29,698 | 1 | 4 | 1 |
| 3149 | Colin Crescent | 2000 | 47 | 2 | MICRO SEAL | \$ 3,594 | 1 | 3 | 1 |
| 314A | Colin Crescent | 2000 | 60 | 2 | MICRO SEAL | \$ 4,588 | 1 | 3 | 1 |
| 314B | Colin Crescent | 2000 | 69 | 2 | MICRO SEAL | \$ 5,276 | 1 | 3 | 1 |
| 314C | Colin Crescent | 2000 | 81 | 2 | MICRO SEAL | \$ 6,193 | 1 | 3 | 1 |
| 314D | Colin Crescent | 2000 | 52 | 2 | MICRO SEAL | \$ 3,976 | 1 | 3 | 1 |
| 314E | Pyramid Lk Rd 500 Block - Bonh | 1977 | 159 | 1 | MICRO SEAL | \$ 11,448 | 1 | 3 | 1 |
| 314F | Pyramid Lk Rd 300 Block - Bonh | 1977 | 159 | 1 | MICRO SEAL | \$ 11,448 | 1 | 3 | 1 |
| 3150 | Elm Ave | 1991 | 51 | 1 | MICRO SEAL | \$ 3,672 | 1 | 3 | 1 |
| 3151 | Elm Ave | 1991 | 50 | 2 | MICRO SEAL | \$ 3,862 | 1 | 3 | 2 |
| 3152 | Elm Ave. between Turret and Bo | 1991 | 160 | 1 | MICRO SEAL | \$ 11,520 | 1 | 3 | 1 |
| 3153 | Patricia Street | 1991 | 105 | 4 | MICRO SEAL | \$ 9,362 | 1 | 3 | 1 |
| 315E | Pyramid Lk Rd 600 Block - Bonh | 1977 | 72 | 1 | MICRO SEAL | \$ 5,184 | 1 | 4 | 1 |
| 3161 | Pyramid Lk Rd 700 Block - Bonh | 1977 | 189 | 1 | MICRO SEAL | \$ 22,680 | 1 | 5 | 1 |
| 3162 | Maligne Ave | 2007 | 117 | 1 | MICRO SEAL | \$ 8,491 | 1 | 4 | 1 |
| 3163 | Turret Street | 2003 | 185 | 5 | MICRO SEAL | \$ 16,453 | 1 | 3 | 1 |
| 3165 | Pyramid LkRd 800 Block North - | 1977 | 77 | 1 | MICRO SEAL | \$ 9,240 | 1 | 5 | 1 |
| 3166 | Pyramid Lk Rd 800 Block South | 1977 | 122 | 1 | MICRO SEAL | \$ 8,901 | 1 | 3 | 1 |
| 316E | Pine Ave | 1977 | 58 | 1 | MICRO SEAL | \$ 4,176 | 1 | 4 | 1 |
| 316F | Pine Ave | 1977 | 45 | 1 | MICRO SEAL | \$ 3,257 | 1 | 4 | 1 |
| 317C | Geikie Street 500 Block | 1991 | 198 | 1 | MICRO SEAL | \$ 21,859 | 1 | 4 | 1 |
| 3187 | Larch Ave | 2007 | 38 | 2 | MICRO SEAL | \$ 3,583 | 1 | 3 | 1 |
| 318B | Pine Ave | 1977 | 58 | 1 | MICRO SEAL | \$ 4,209 | 1 | 4 | 1 |
| 318C | Pine Ave | 1977 | 43 | 1 | MICRO SEAL | \$ 3,121 | 1 | 4 | 1 |
| 3190 | Pine Ave | 1977 | 44 | 1 | MICRO SEAL | \$ 3,222 | 1 | 3 | 1 |
| 3191 | Geikie Street 800 Block | 1991 | 240 | 1 | MICRO SEAL | \$ 63,360 | 1 | 5 | 1 |
| 3192 | Geikie Street 800 Block South | 1994 | 85 | 1 | MICRO SEAL | \$ 10,304 | 1 | 5 | 1 |
| 3193 | Geikie Street 700 Block North | 1991 | 201 | 1 | MICRO SEAL | \$ 24,224 | 1 | 5 | 1 |
| 3194 | Geikie Street 600 Block | 1991 | 145 | 1 | MICRO SEAL | \$ 24,471 | 1 | 5 | 1 |
| 3195 | Hazel Ave | 2002 | 45 | 1 | MICRO SEAL | \$ 3,240 | 1 | 3 | 1 |
| 3196 | Hazel Ave | 2002 | 43 | 1 | MICRO SEAL | \$ 3,096 | 1 | 3 | 1 |
| 3197 | Patricia Street 600 Block Nort | 1991 | 52 | 1 | MICRO SEAL | \$ 3,744 | 1 | 3 | 1 |

Table 8 – Five-Year (2017-2021) Roadways Micro-Surfacing Segment Listing

| SEGMENT ID | LOCATION | AGE | LENGTH | YEAR | TREATMENT | COST | Fatigue Cracking | Surface Condition | Lineal Cracking |
|-------------------|--------------------------------|------------|---------------|-------------|------------------|-------------|-------------------------|--------------------------|------------------------|
| 3198 | Patricia Street 600 Block Sout | 1991 | 222 | 1 | MICRO SEAL | \$ 15,984 | 1 | 3 | 1 |
| 3199 | Hazel Ave | 2002 | 54 | 5 | MICRO SEAL | \$ 4,802 | 1 | 3 | 1 |
| 319A | Spruce Ave Roadway Surfaces | 2000 | 48 | 1 | MICRO SEAL | \$ 3,456 | 1 | 4 | 1 |
| 319B | Spruce Ave | 2000 | 46 | 1 | MICRO SEAL | \$ 3,327 | 1 | 4 | 1 |
| 319C | Spruce Ave | 2000 | 44 | 1 | MICRO SEAL | \$ 3,176 | 1 | 3 | 1 |
| 319D | Spruce Ave | 2000 | 50 | 1 | MICRO SEAL | \$ 3,610 | 1 | 3 | 1 |
| 319F | Pine Ave. between Patricia and | 1975 | 44 | 1 | MICRO SEAL | \$ 3,168 | 1 | 3 | 1 |
| 31A0 | Pine Ave | 1975 | 43 | 1 | MICRO SEAL | \$ 3,113 | 1 | 3 | 1 |
| 31A1 | Pine Ave | 1974 | 44 | 1 | MICRO SEAL | \$ 3,168 | 1 | 3 | 1 |
| 31AA | Patricia Street 700 Block | 1991 | 371 | 1 | MICRO SEAL | \$ 44,734 | 1 | 4 | 1 |
| 31AB | Patricia Street 800 Block | 1991 | 209 | 1 | MICRO SEAL | \$ 16,177 | 2 | 3 | 1 |
| 31AE | Geikie Street 900 Block North | 1994 | 134 | 1 | MICRO SEAL | \$ 9,648 | 1 | 3 | 1 |
| 31AF | Geikie Street 900 Block South | 1994 | 56 | 1 | MICRO SEAL | \$ 4,032 | 1 | 3 | 1 |
| 31B2 | Patricia Street 900 Block | 1991 | 190 | 1 | MICRO SEAL | \$ 14,095 | 3 | 3 | 2 |
| 31B3 | Willow Ave | 1997 | 46 | 1 | MICRO SEAL | \$ 3,312 | 1 | 3 | 1 |
| 31B4 | Willow Ave | 1997 | 144 | 3 | MICRO SEAL | \$ 11,547 | 1 | 3 | 1 |
| 31B5 | Pyramid Lk Rd 900 Block | 1977 | 245 | 1 | MICRO SEAL | \$ 29,903 | 1 | 4 | 2 |
| 31BE | Patricia Ave 1000 East | 1979 | 202 | 1 | MICRO SEAL | \$ 33,936 | 1 | 5 | 1 |
| 31BF | Patricia street 1000 middle | 2010 | 84 | 1 | MICRO SEAL | \$ 14,112 | 1 | 5 | 1 |
| 31C3 | Patricia Crescent | 1980 | 45 | 4 | MICRO SEAL | \$ 3,814 | 1 | 3 | 1 |
| 31C4 | Brewster Crescent | 1980 | 36 | 2 | MICRO SEAL | \$ 2,768 | 1 | 3 | 1 |
| 31C5 | Patricia Crescent | 1980 | 397 | 4 | MICRO SEAL | \$ 33,651 | 1 | 3 | 1 |
| 31CF | Connaught Dr West | 1981 | 262 | 2 | MICRO SEAL | \$ 40,062 | 1 | 3 | 1 |
| 31D0 | Connaught Dr West to Sleepy Ho | 1981 | 23 | 2 | MICRO SEAL | \$ 3,517 | 1 | 3 | 1 |
| 31D2 | Hwy 93A | 2000 | 95 | 4 | MICRO SEAL | \$ 8,059 | 1 | 3 | 1 |
| 31D3 | Hwy 93A Rail Crossing | 2000 | 32 | 5 | MICRO SEAL | \$ 2,846 | 1 | 3 | 1 |
| 31D4 | Hwy 93A | 2000 | 50 | 5 | MICRO SEAL | \$ 4,447 | 1 | 3 | 1 |
| 31D5 | Hwy 93A | 2000 | 98 | 5 | MICRO SEAL | \$ 8,715 | 1 | 3 | 1 |
| 31D8 | S Block | 1972 | 81 | 1 | MICRO SEAL | \$ 6,610 | 1 | 3 | 1 |
| 31D9 | S Block | 1972 | 222 | 2 | MICRO SEAL | \$ 16,973 | 1 | 3 | 1 |
| 31DA | S Block | 1972 | 495 | 1 | MICRO SEAL | \$ 59,400 | 1 | 4 | 1 |
| 31DB | Compound Road | 1972 | 136 | 1 | MICRO SEAL | \$ 9,792 | 1 | 4 | 1 |
| 31DD | Compound road | 1972 | 128 | 1 | MICRO SEAL | \$ 46,080 | 1 | 5 | 1 |
| 31DF | Compound road | 1972 | 198 | 1 | MICRO SEAL | \$ 52,272 | 1 | 5 | 1 |
| 3203 | Tonquin Street | 1977 | 134 | 1 | MICRO SEAL | \$ 12,221 | 1 | 4 | 1 |
| 3204 | Turret Street | 1996 | 131 | 1 | MICRO SEAL | \$ 9,432 | 1 | 3 | 1 |
| 37BD | Alley between Geikie and Patri | 1955 | 151 | 1 | MICRO SEAL | \$ 10,570 | 1 | 4 | 1 |
| 38CB | Elm Ave | 1991 | 44 | 1 | MICRO SEAL | \$ 3,168 | 1 | 3 | 1 |
| 38E7 | Pine Ave | 1975 | 44 | 1 | MICRO SEAL | \$ 3,185 | 1 | 3 | 1 |
| 391B | Connaught Dr Sawridge | 1981 | 251 | 4 | MICRO SEAL | \$ 42,604 | 1 | 3 | 1 |
| 3924 | Cedar Ave | 1991 | 43 | 2 | MICRO SEAL | \$ 3,296 | 1 | 3 | 1 |
| 3926 | Cedar Ave Roadway Surfaces | 1991 | 46 | 2 | MICRO SEAL | \$ 3,526 | 1 | 3 | 1 |
| 3927 | Pyramid Ave | 1997 | 78 | 1 | MICRO SEAL | \$ 20,622 | 1 | 5 | 1 |
| 3928 | Pyramid Ave | 1997 | 64 | 1 | MICRO SEAL | \$ 16,950 | 1 | 5 | 1 |
| 3929 | Pyramid Ave | 1997 | 86 | 1 | MICRO SEAL | \$ 22,737 | 1 | 5 | 1 |
| 392B | Connaught Dr 400 Block | 1981 | 145 | 2 | MICRO SEAL | \$ 27,757 | 1 | 3 | 1 |
| 392F | Connaught Dr 700 Block | 1981 | 367 | 4 | MICRO SEAL | \$ 62,210 | 1 | 3 | 1 |
| 3930 | Connaught Dr 800 Block | 1981 | 208 | 4 | MICRO SEAL | \$ 35,258 | 1 | 3 | 1 |
| 3931 | Connaught Dr 900 Block | 1981 | 265 | 2 | MICRO SEAL | \$ 40,520 | 1 | 3 | 1 |
| 3932 | Miette Ave | 1994 | 87 | 1 | MICRO SEAL | \$ 12,528 | 1 | 3 | 1 |
| 3934 | Miette Ave | 1994 | 64 | 1 | MICRO SEAL | \$ 9,216 | 1 | 3 | 1 |
| 3935 | Miette Ave | 1994 | 66 | 1 | MICRO SEAL | \$ 9,504 | 1 | 3 | 1 |
| 3936 | Miette Ave | 1994 | 65 | 1 | MICRO SEAL | \$ 9,360 | 1 | 3 | 1 |
| 3937 | Miette Ave | 1994 | 117 | 1 | MICRO SEAL | \$ 21,386 | 1 | 4 | 1 |
| 393D | Miette Ave | 1994 | 213 | 1 | MICRO SEAL | \$ 112,464 | 1 | 5 | 1 |
| 393E | Miette Ave | 1994 | 29 | 1 | MICRO SEAL | \$ 4,176 | 1 | 3 | 1 |
| 3940 | Hazel Ave | 2002 | 54 | 5 | MICRO SEAL | \$ 4,802 | 1 | 3 | 1 |
| 3941 | Pyramid Ave | 1997 | 67 | 1 | MICRO SEAL | \$ 17,744 | 1 | 5 | 1 |
| 3946 | Bonhomme St 200 Block North | 1970 | 56 | 1 | MICRO SEAL | \$ 6,742 | 1 | 4 | 1 |

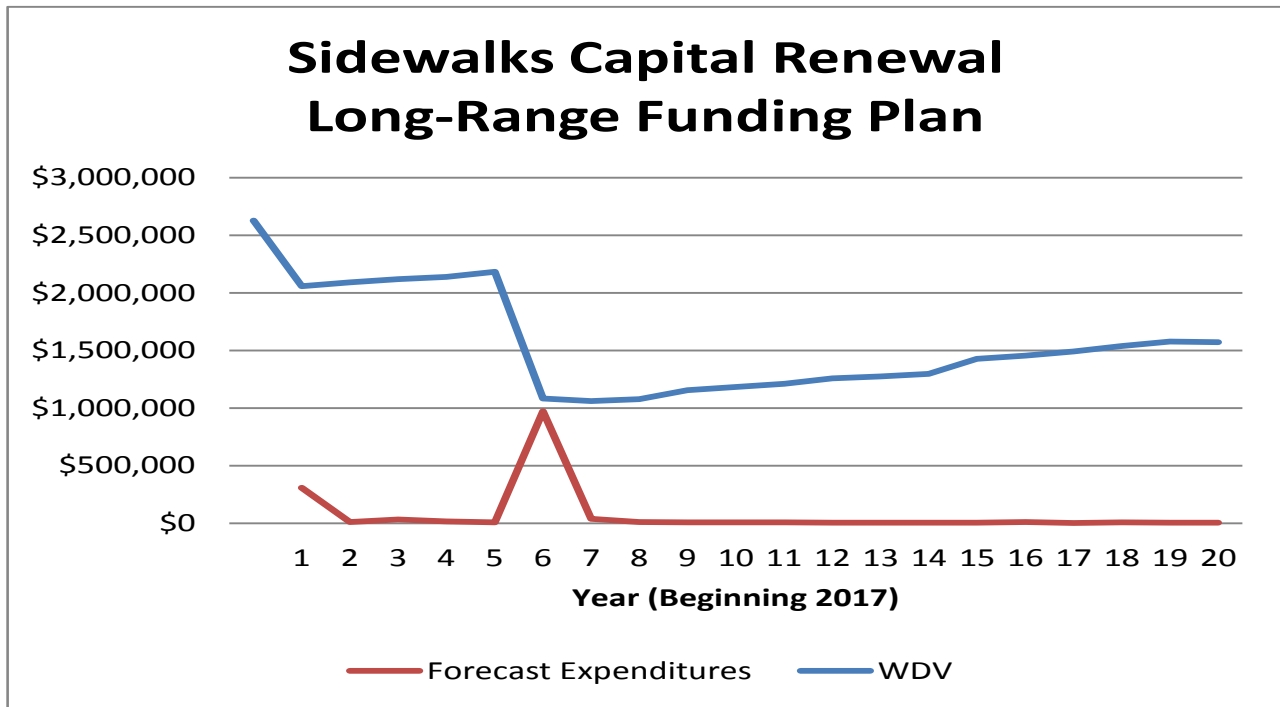
Table 9 – Five-Year (2017-2021) Roadways Resurfacing Segment Listing

| SEGMENT ID | LOCATION | AGE | LENGTH | YEAR | TREATMENT | COST | Fatigue Cracking | Surface Condition | Lineal Cracking |
|-------------------|-----------------------|------------|---------------|-------------|------------------|-------------|-------------------------|--------------------------|------------------------|
| 311B | Balsam Ave | 2000 | 44 | 1 | RESURFACE | \$ 21,120 | 5 | 4 | 1 |
| 311C | Balsam Ave | 2000 | 50 | 1 | RESURFACE | \$ 24,000 | 5 | 4 | 1 |
| 31DC | Compound road | 1972 | 388 | 1 | RESURFACE | \$186,240 | 1 | 5 | 1 |
| 38C8 | Aspen Crescent | 1975 | 345 | 1 | RESURFACE | \$165,600 | 5 | 3 | 1 |
| 38E6 | 80 Block Geikie | 1975 | 377 | 2 | RESURFACE | \$180,960 | 4 | 3 | 1 |
| 391C | Connaught Dr 90 Block | 1981 | 242 | 2 | RESURFACE | \$232,320 | 4 | 4 | 2 |

4.2 Sidewalks (Concrete)

As presented earlier in the State of the Infrastructure section, spalling is the primary condition distress. As a result, the optimal scenario is initiating surface treatment in the form of bonded overlays and diamond grinding & thin-bituminous-surfacing in the short-term (5-year) horizon. The following figure summarizes the projected spending over the 20-year horizon. This is indicating a spike of renewal in year 6.

Figure 18 – Sidewalk Long-Range Funding Projection



The following table measures the effectiveness of the proposed spending in the short-term (i.e. 5-year) and long-term (i.e. 20-year) horizons. In the short-term the Municipality should be in a position to invest approximately \$75,000/year. With this spending is a positive improvement in the sidewalk condition state estimated at 3-4%/year in the short term and 2%/year in the long term. This is indicative of the ROII, which demonstrates a significantly greater improvement in the asset value given the maintenance and renewal investment.

Table 10 – Sidewalk Performance Summary

| Sidewalks | Optimization 5-year | Optimization 20-year |
|-----------------------------------|--------------------------------|---------------------------------|
| Program Expenditures | | |
| - Total Period Expenditure (M\$) | \$0.375 | \$1.475 |
| - Annualized Expenditure (M\$/yr) | \$0.075 | \$0.074 |
| Monetary Performance | | |
| - WDV (initial) | \$2.628 | \$2.628 |
| - WDV (End) | \$2.184 | \$1.571 |
| Improved Perf (+) | \$0.444 | \$1.057 |
| Annual Perf Change (M\$/yr) | \$0.089 | \$0.053 |
| | 3% | 2% |
| Condition State/Index | | |
| - Index (initial) | 21 | 21 |
| - Index (End) | 17 | 14 |
| Improved Condition (+) | 4 | 7 |
| Annual Condition Change (/yr) | 0.8 | 0.4 |
| Annual Condition Change (%/yr) | 4% | 2% |
| ROI - Annualized (%/yr) | 218% | 172% |
| - (100% = Stabilization Level) | | |

Within the five-year horizon, the following table summarizes the annualized cost broken out by maintenance (i.e. patching and grinding), bonded overlays, and grinding & thin-bituminous-surfacing (also referred to as micro-surfacing). It is a relatively light program. However, as this is a relatively new approach to the Municipality, it may be considered a trial period. Year six is projecting more need for sidewalk renewal, which is not included in the expenditure estimates below. Due to the well maintained sidewalk state, the need for trip-edge grinding is relatively insignificant. There still may be need for some unexpected surface maintenance services.

Table 11 – Sidewalk Five-Year (2017-2021) Maintenance and Renewal Summary

| Treatment Activity | Length (m) | Cost (\$/yr) |
|--|-------------------|---------------------|
| Maintenance (Grinding and Patching) | | \$0 |
| Bonded Overlay | 2,548 | \$12,000 |
| Diamond Grinding & Thin-Bituminous-Surface | 2,648 | \$13,000 |
| Average Annual Cost | | \$75,000 |
| | | |
| Total Cost over 5-Years | | \$375,000 |

In our lifecycle optimization analysis, we are bringing forward two treatments the Municipality may not be familiar with. One is a bonded overlay, which is a thin epoxy that bonds to the existing concrete. It can be applied to the entire sidewalk surface. However, we bring this treatment in as a spot repair of cracks and spalling. It works well for both the sidewalk surface and curb repairs.

Figure 19 – Bonded Overlay



Then we bring forward diamond grinding of the major cracks to level the surface followed by application of a thin-bituminous-surface indicative of a micro-surface. This we apply to the full length of the sidewalk. The ascetic quality of the diamond grinding & thin-bituminous-surface below is better than the bonded overlay above. However, it does cost more and not appropriate for smaller spot repairs. The diamond grinding & thin-bituminous-surface treatment will complement well with the roadways (streets) micro-surfacing program; where the works for roadways and sidewalks could be tendered in a single contract.

Figure 20 – Diamond Grinding and Thin-Bituminous-Surface



The following tables list the specific sidewalk segments targeted for bonded overlay and grinding & thin-bituminous-surface treatments in the five-year horizon. Included is the condition state for the existing and modelled condition for each of the major distresses. The condition rating score is as follows:

- 1 – very good
- 2 – good
- 3 – fair
- 4 – poor
- 5 – very poor

The following table lists the recommended bonded overlay sections. The triggering condition distress is spalling. However on application of the treatment, cracked areas would also be mitigated. The intent is to apply over the more moderate to major distress areas in spot locations. The sidewalks triggered for this treatment are in the fair condition state “3” for spalling.

Table 12 – Five-Year (2017-2021) Sidewalk Bonded Overlay Segment Listing

| <u>ITEM</u> | <u>SEGMENT_ID</u> | <u>AGE</u> | <u>LENGTH (m)</u> | <u>YEAR</u> | <u>TREATMENT</u> | <u>COST</u> | <u>Cracking</u> | <u>Spalling</u> |
|-------------|-------------------|------------|-------------------|-------------|------------------|-------------|-----------------|-----------------|
| SIDEWALK | 326D | 1974 | 208 | 4 | BONDED OLAY | \$ 1,297 | 1 | 2 |
| SIDEWALK | 326D | 1974 | 208 | 3 | BONDED OLAY | \$ 6,196 | 2 | 3 |
| SIDEWALK | 327F | 1974 | 180 | 3 | BONDED OLAY | \$ 4,485 | 1 | 3 |
| SIDEWALK | 3284 | 1975 | 220 | 4 | BONDED OLAY | \$ 1,118 | 1 | 2 |
| SIDEWALK | 328E | 1975 | 132 | 2 | BONDED OLAY | \$ 5,218 | 1 | 3 |
| SIDEWALK | 32AB | 1977 | 342 | 1 | BONDED OLAY | \$ 966 | 1 | 3 |
| SIDEWALK | 32DF | 1980 | 70 | 3 | BONDED OLAY | \$ 4,114 | 1 | 3 |
| SIDEWALK | 32E2 | 1980 | 135 | 2 | BONDED OLAY | \$ 5,311 | 2 | 3 |
| SIDEWALK | 333B | 1991 | 30 | 5 | BONDED OLAY | \$ 1,772 | 1 | 3 |
| SIDEWALK | 3355 | 1994 | 72 | 3 | BONDED OLAY | \$ 1,665 | 2 | 3 |
| SIDEWALK | 3358 | 1994 | 158 | 5 | BONDED OLAY | \$ 3,785 | 1 | 3 |
| SIDEWALK | 3365 | 1994 | 74 | 3 | BONDED OLAY | \$ 3,640 | 1 | 3 |
| SIDEWALK | 17N16 | | 90 | 3 | BONDED OLAY | \$ 5,889 | 1 | 3 |
| SIDEWALK | 17N32 | | 55 | 5 | BONDED OLAY | \$ 884 | 1 | 3 |
| SIDEWALK | 17N46 | | 100 | 4 | BONDED OLAY | \$ 4,020 | 1 | 3 |
| SIDEWALK | 17N55 | | 58 | 4 | BONDED OLAY | \$ 2,323 | 2 | 3 |

The following table lists the recommended diamond grinding & thin-bituminous-surface sections. The triggering condition distress is spalling. However on application of the treatment, cracked areas would also be mitigated through the continuous application of the treatment over the entire length of sidewalk. It would be a complete renewal. The sidewalks triggered for this treatment are in the very poor condition state "5" for spalling.

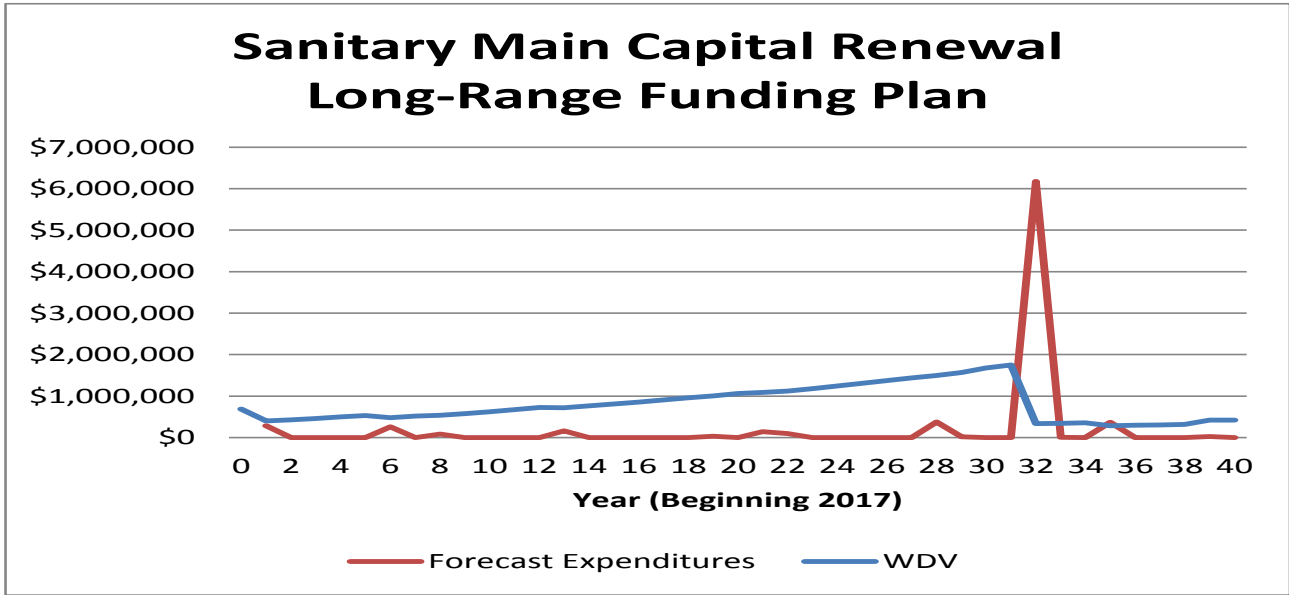
Table 13 – Five-Year (2017-2021) Sidewalk Diamond Grinding and Thin-Bituminous-Surface Segment Listing

| <u>ITEM</u> | <u>SEGMENT ID</u> | <u>AGE</u> | <u>LENGTH (m)</u> | <u>YEAR</u> | <u>TREATMENT</u> | <u>COST</u> | <u>Cracking</u> | <u>Spalling</u> |
|-------------|-------------------|------------|-------------------|-------------|------------------|-------------|-----------------|-----------------|
| SIDEWALK | 326A | 1974 | 85 | 1 | GRIND TBS | \$ 10,710 | 1 | 5 |
| SIDEWALK | 327C | 1974 | 75 | 1 | GRIND TBS | \$ 9,450 | 2 | 5 |
| SIDEWALK | 3281 | 1974 | 130 | 1 | GRIND TBS | \$ 16,380 | 1 | 5 |
| SIDEWALK | 328A | 1975 | 87 | 1 | GRIND TBS | \$ 10,990 | 1 | 5 |
| SIDEWALK | 3311 | 1991 | 33 | 1 | GRIND TBS | \$ 4,130 | 1 | 5 |
| SIDEWALK | 3326 | 1991 | 30 | 1 | GRIND TBS | \$ 3,780 | 2 | 5 |
| SIDEWALK | 3329 | 1991 | 70 | 1 | GRIND TBS | \$ 8,820 | 1 | 5 |
| SIDEWALK | 3334 | 1991 | 47 | 1 | GRIND TBS | \$ 5,950 | 2 | 5 |
| SIDEWALK | 3336 | 1991 | 47 | 1 | GRIND TBS | \$ 5,950 | 2 | 5 |
| SIDEWALK | 335D | 1994 | 190 | 1 | GRIND TBS | \$ 23,940 | 1 | 5 |
| SIDEWALK | 3371 | 1994 | 94 | 1 | GRIND TBS | \$ 11,830 | 1 | 5 |
| SIDEWALK | 17N12 | | 39 | 1 | GRIND TBS | \$ 5,460 | 1 | 5 |
| SIDEWALK | 17N15 | | 32 | 1 | GRIND TBS | \$ 4,480 | 1 | 5 |
| SIDEWALK | 17N22 | | 31 | 1 | GRIND TBS | \$ 2,590 | 1 | 5 |
| SIDEWALK | 17N31 | | 115 | 1 | GRIND TBS | \$ 9,660 | 1 | 5 |
| SIDEWALK | 17N39 | | 323 | 1 | GRIND TBS | \$ 27,160 | 1 | 5 |
| SIDEWALK | 17N43 | | 208 | 1 | GRIND TBS | \$ 26,180 | 1 | 5 |
| SIDEWALK | 17N44 | | 130 | 1 | GRIND TBS | \$ 16,380 | 1 | 5 |
| SIDEWALK | 17N45 | | 54 | 4 | GRIND TBS | \$ 6,790 | 1 | 5 |
| SIDEWALK | 17N47 | | 90 | 1 | GRIND TBS | \$ 11,340 | 1 | 5 |
| SIDEWALK | 17N48 | | 70 | 1 | GRIND TBS | \$ 8,820 | 2 | 5 |
| SIDEWALK | 17N50 | | 285 | 1 | GRIND TBS | \$ 35,910 | 1 | 5 |
| SIDEWALK | 17N51 | | 50 | 1 | GRIND TBS | \$ 6,300 | 1 | 5 |
| SIDEWALK | 17N52 | | 130 | 1 | GRIND TBS | \$ 16,380 | 1 | 5 |
| SIDEWALK | 17N53 | | 58 | 1 | GRIND TBS | \$ 7,280 | 2 | 5 |
| SIDEWALK | 17N57 | | 145 | 1 | GRIND TBS | \$ 18,270 | 2 | 5 |

4.3 Wastewater Collection (Sanitary, Storm, Manholes and Catch Basins)

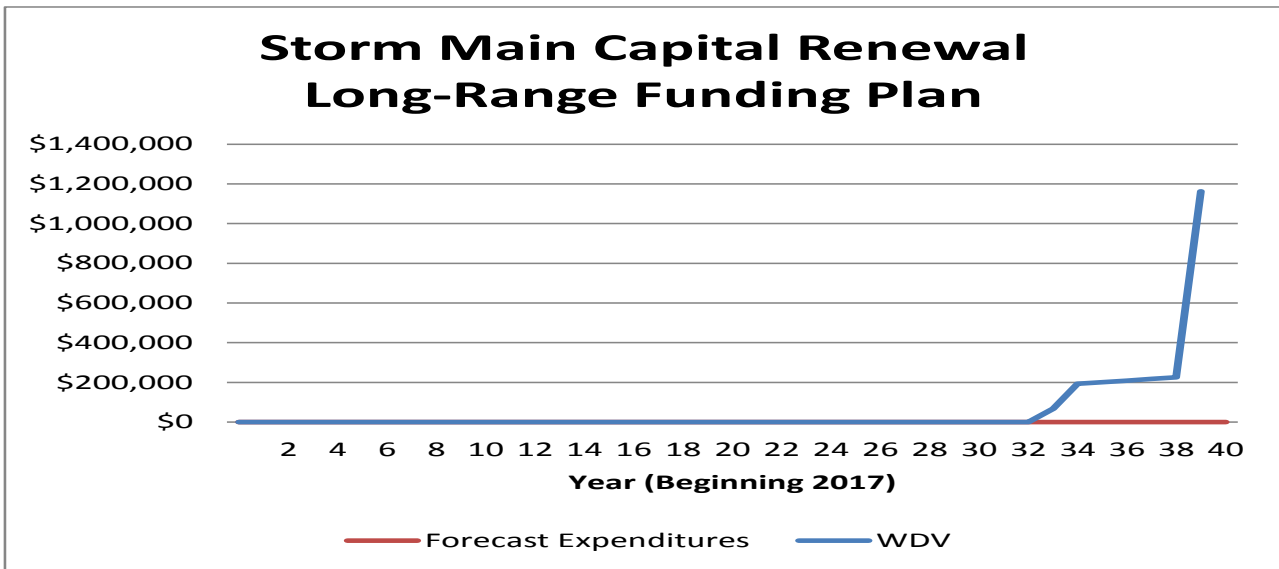
Even though the sanitary sewer system is in fairly good condition, there are some pipes subject to renewal needs in the short term. There are also some pipes in need of maintenance due to root build up. However, the greater renewal expenditure needs are forecasted for thirty years from today (i.e. 2047). The following figure illustrates the Sanitary Mains long-range expenditure forecast.

Figure 21 – Sanitary Mains Long-Range Funding Projection



Alternatively, the storm sewer system is currently in very good condition and forecasted to remain in fairly good condition for the long-range horizon. As shown in the following figure, the condition starts showing noticeable deterioration in approximately 30 years from today (i.e. 2047); with expenditure needs likely to follow in approximately 40 years from today (i.e. 2057).

Figure 22 – Storm Mains Long-Range Funding Projection



The projection is similar for the sanitary and storm manholes and catch basins, with negligible capital renewal expenditure needs within the long-range (40 year) horizon. The wastewater system capital renewal program will be predominately the sanitary mains infrastructure group.

In addition to the above noted costs, the cost projections include repair of sanitary service connection deficiencies identified by Municipal Operations personnel. It is estimated that there is a deficiency to approximately 50 sanitary service connections causing sewer back-up. This may be remedied by cured in place pipe (CIPP) lining of the service connection pipes conducted during a time when other CIPP works are being completed.

The following table measures the effectiveness of the proposed spending in the short-term (i.e. 6-year) and long-term (40-year) horizons for the sanitary mains. A six-year assessment was used instead of five, due to some expenditure needs determined for year 6 that we wanted to capture. Based on the treatment scheduling, the results are indicating a 3-6%/yr short-term and 1-2% long-term improvement to the condition of the sanitary sewer mains. This was measured by the physical condition index along with the monetary WDV. Both are relatively consistent. The ROII, which indicates the asset improvement above sustainability level, is modest but positive.

Table 14 – Sanitary Sewer Mains Performance Summary

| Sanitary Mains | Optimization 6-year | Optimization 40-year |
|-----------------------------------|--------------------------------|---------------------------------|
| Program Expenditures | | |
| - Total Period Expenditure (M\$) | \$0.636 | \$8.027 |
| - Annualized Expenditure (M\$/yr) | \$0.106 | \$0.201 |
| Monetary Performance | | |
| - WDV (initial) | \$0.685 | \$0.685 |
| - WDV (Final) | \$0.476 | \$0.417 |
| Improved Perf (+) | \$0.209 | \$0.268 |
| Annual Perf Change (\$/yr) | \$0.035 | \$0.007 |
| | 5% | 1% |
| Condition State/Index | | |
| - Index (initial) | 6 | 6 |
| - Index (Final) | 5 | 2 |
| Improved Condition (+) | 1 | 4 |
| Annual Condition Change (/yr) | 0.2 | 0.1 |
| Annual Condition Change (%/yr) | 3% | 2% |
| ROII - Annualized (%/yr) | 133% | 103% |
| - (100% = Stabilization Level) | | |

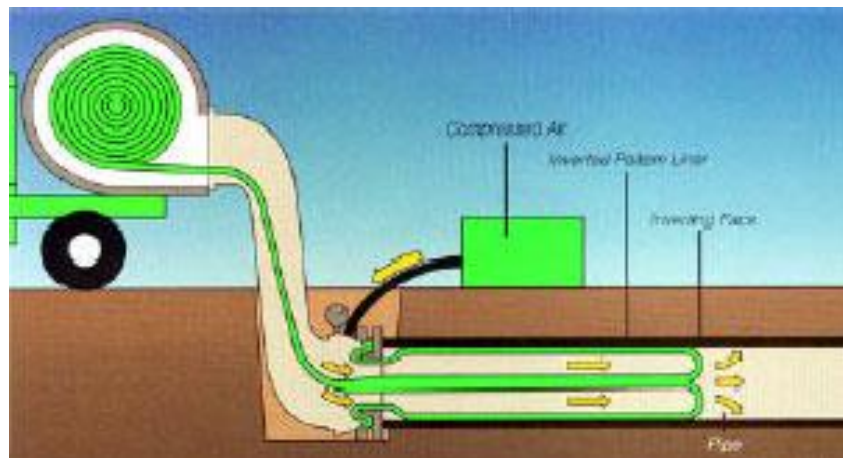
Within the six-year horizon, the following table summarizes the annualized cost broken out by maintenance (i.e. rooting & jetting), cured in place lining and pipe bursting. Cured in place lining is lower unit cost than pipe bursting. However, if the pipe is too far deteriorated, or undersized, the pipe may have to be replaced. The unit price differences between pipe bursting and open cut excavation are relatively negligible. For the purpose of this report, we assume pipe bursting over open cut excavation due to less surface disruption to the community. The following table includes also the costs for lining of one manhole identified in for renewal in the six-year horizon.

Table 15 – Sanitary Sewer Six-Year (2017-2022) Maintenance and Renewal Summary

| <u>Treatment Activity</u> | <u>Length (m)</u> | <u>Cost (\$/yr)</u> |
|-----------------------------------|-------------------|---------------------|
| Rooting & Jetting | 399 | \$2,000 |
| Repair of SC Deficiencies (CIPP) | 50 | \$15,000 |
| Cured in Place Pipe (CIPP) Liners | 582 | \$58,000 |
| Pipe Bursting | 200 | \$31,000 |
| Average Annual Cost | | \$106,000 |
| | | |
| Total Cost Over 6 Years | | \$636,000 |

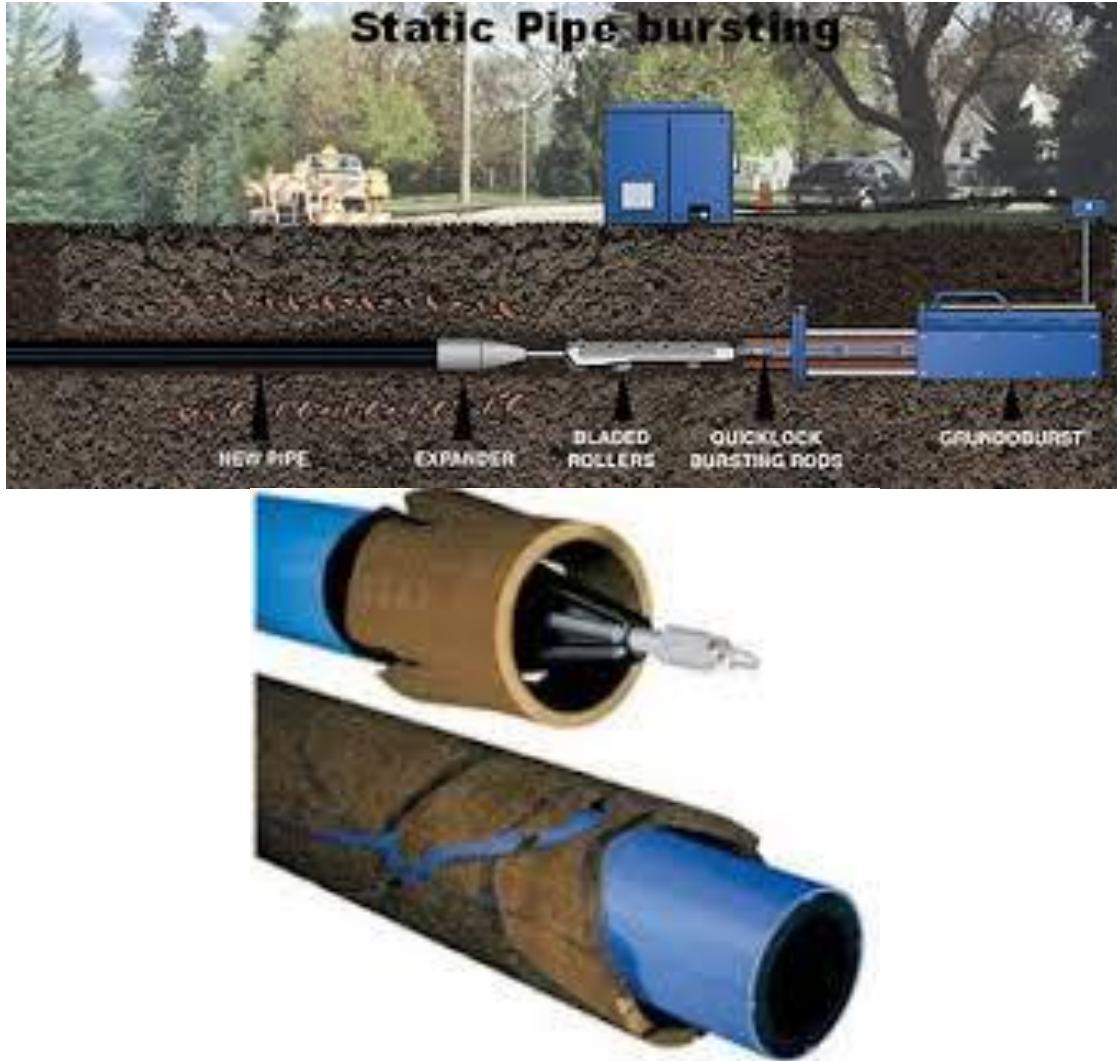
The following figure provides illustrations of cured in place pipe (CIPP) lining technique. It is a trenchless technology designed to minimize disruption to the surface during application. Note the top picture describes the process of inserting the fibreglass material. Then the fibreglass sock is expanded in the pipe with the injection water or hot air. The photo underneath illustrate a moderate level crack within pipe segment JSD011 and the result of what the completed CIPP liner would look like within the pipe.

Figure 23 – Cured In-Place Liners



The following figure provides illustrations of pipe-bursting technique. It too is a trenchless technology designed to minimize disruption to the surface during application. It works by pulling through a flexible HDPE pipe while busting the existing pipe as the new pipe is being pulled through. The top picture illustrates the process, while the bottom picture illustrates what the new pipe looks like relative to the old.

Figure 24 – Pipe Bursting



The following tables list the specific sanitary pipe segments targeted for Operations & Maintenance (i.e. rooting and jetting) and major works (i.e. lining and replacement). Included is the condition state for the existing and modelled condition for each of the major distresses. The condition rating score is as follows:

- 1 – very good
- 2 – good
- 3 – fair
- 4 – poor
- 5 – very poor

Each of the tables also includes a “TESTED” column. A “TRUE” indicates the pipe was physically inspected. As it was unfeasible to provide physical inspections of all the pipes, those not inspected, the condition assessment was correlated based on age and material type in comparison to pipes inspected of similar nature.

The following table lists the recommended O&M sections. Tree root infiltration through the pipe joints was the major issue on these pipes. The following figure illustrates CCTV footage of pipe segment JSD039, indicating the rooting issue that requires maintenance works to address.

Figure 25 - O&M Trigger for Sanitary Pipe #JSD039



Table 16 – Six-Year (2017-2022) Operation and Maintenance Segment Listing

| ITEM | SEGMENT | LOCATION | AGE | LENGTH | DIA | MATERIAL | TESTED | YEAR | TREATMENT | COST | Structural | O&M | Capacity |
|--------------|---------|--------------------------------|------|--------|-----|----------|--------|------|-----------|----------|------------|-----|----------|
| SANITRY MAIN | JSD009 | Pyramid Lk Rd between Elm & Py | 1973 | 83 | 203 | VCT | TRUE | 1 | ROOT_JET | \$ 2,311 | 1 | 5 | 3 |
| SANITRY MAIN | JSD038 | Between Activity Center and Sc | 1973 | 31 | 203 | VCT | TRUE | 1 | ROOT_JET | \$ 1,100 | 1 | 5 | 1 |
| SANITRY MAIN | JSD039 | Cedar Ave | 1972 | 46 | 203 | CONCRETE | TRUE | 1 | ROOT_JET | \$ 1,595 | 1 | 5 | 1 |
| SANITRY MAIN | JSD040 | Geikie Street between Pyramid | 1973 | 35 | 203 | VCT | TRUE | 1 | ROOT_JET | \$ 1,210 | 1 | 5 | 1 |
| SANITRY MAIN | NE007 | Juniper Street | 1955 | 85 | 203 | VCT | TRUE | 1 | ROOT_JET | \$ 2,311 | 1 | 5 | 3 |
| SANITRY MAIN | RW005 | Alley between Connaught & Patr | 1955 | 119 | 203 | VCT | TRUE | 1 | ROOT_JET | \$ 3,143 | 2 | 5 | 1 |

The following table lists the recommended major works sections based on the structural condition. Where the pipe is not broken, liners are a viable alternative. Where the pipe has undergone further deterioration and the pipe is no longer structurally intact, the treatment alternative moves to pipe replacement. In this case, we are showing pipe replacement by pipe bursting. It could be completed by open cut excavation as well. The costs are comparable. It is possible that significantly deteriorated pipes could undergo a lining treatment, even late in its remaining service life. However, that would require excavation and repair of the broken sections prior to initiating the trenchless liner technique. In this case our recommended liner is referred to as cured in place pipe (CIPP). The following figure illustrates CCTV footage of pipe segment JSD012, indicating the structural conditions triggering treatment through lining. Note the two types of cracking (longitudinal and spiral). The pipe is functional today. However, through its lifecycle if the deficiency is deferred too long, the cost to replace the pipe is approximately four-times the cost of lining.

Figure 26 - Structural Condition Trigger for CIPP on Sanitary Pipe #JSD012



The following figure illustrates CCTV footage of pipe segment JSD023 indicating the structural conditions triggering replacing the pipe through pipe bursting. In this case the circumference cracking is accelerating (left) in places to which the pipe is beginning to fail (right) in places. As a result, the pipe needs to be replaced. Lining is only an option on excavation and repair of the broken sections, which has been calculated in the model to be less cost effective than pipe replacement.

Figure 27 - Structural Condition Trigger for Pipe Bursting (Replacement) on Sanitary Pipe #JSD023



It should be noted that the structural condition triggering pipe lining through CIPP techniques is in the range of fair “3” to poor “4”. The structural condition triggering pipe replacement through bursting techniques is very poor “5”.

The following table shows one manhole. The treatment generated is a spin-cast lining technique. However, if other pipes are being lined through CIPP methods, the CIPP lining technique would too be appropriate for the manhole.

Table 17 – Six-Year (2017-2022) Major Treatment Segment Listing

| <u>ITEM</u> | <u>SEGMENT</u> | <u>LOCATION</u> | <u>AGE</u> | <u>LENGTH</u> | <u>DIA</u> | <u>MATERIAL</u> | <u>TESTED</u> | <u>YEAR</u> | <u>TREATMENT</u> | <u>COST</u> | <u>Structural</u> | <u>O&M</u> | <u>Capacity</u> |
|--------------|----------------|--------------------------------|------------|---------------|------------|-----------------|---------------|-------------|------------------|-------------|-------------------|----------------|-----------------|
| SANITRY MAIN | JSD001 | Pyramid Lk Rd near Maligne Ave | 1973 | 80 | 254 | VCT | TRUE | 1 | CIPP 2 | \$ 32,000 | 3 | 1 | 1 |
| SANITRY MAIN | JSD011 | Pyramid Lk Rd between Elm & Py | 1973 | 79 | 254 | VCT | TRUE | 6 | CIPP 2 | \$ 96,756 | 4 | 1 | 1 |
| SANITRY MAIN | JSD012 | Pyramid Ave between Pyramid Lk | 1972 | 53 | 254 | CONCRETE | TRUE | 1 | CIPP 2 | \$ 21,000 | 3 | 1 | 1 |
| SANITRY MAIN | JSD022 | Pyramid Ave between Colin and | 1973 | 66 | 203 | VCT | TRUE | 1 | CIPP 2 | \$ 21,000 | 4 | 1 | 1 |
| SANITRY MAIN | JSD023 | Pyramid Ave between Colin and | 1973 | 65 | 203 | VCT | TRUE | 1 | PIPE BURST 2 | \$ 50,400 | 5 | 1 | 1 |
| SANITRY MAIN | JSD041 | Geikie Street and Pyramid Ave | 1973 | 26 | 203 | VCT | TRUE | 1 | CIPP 2 | \$ 8,513 | 3 | 1 | 1 |
| SANITRY MAIN | NE012 | Patricia Circle South End | 1966 | 50 | 203 | VCT | TRUE | 6 | CIPP 2 | \$ 66,916 | 4 | 1 | 1 |
| SANITRY MAIN | NE014 | Patricia Circle North End | 1966 | 13 | 203 | VCT | TRUE | 1 | CIPP 2 | \$ 4,000 | 4 | 1 | 1 |
| SANITRY MAIN | NE018 | Patricia Circle North End | 1966 | 27 | 203 | VCT | TRUE | 1 | PIPE BURST 2 | \$ 20,400 | 5 | 1 | 1 |
| SANITRY MAIN | RW003 | Alley between Connaught & Pat | 1955 | 95 | 203 | VCT | FALSE | 6 | CIPP 2 | \$ 42,236 | 3 | 3 | 1 |
| SANITRY MAIN | RW004 | Alley Between Connaught & Patr | 1955 | 109 | 203 | VCT | FALSE | 6 | CIPP 2 | \$ 48,467 | 3 | 3 | 1 |
| SANITRY MAIN | RW010 | Alley between Connaught and Pa | 1955 | 78 | 305 | VCT | TRUE | 1 | PIPE BURST 2 | \$ 90,000 | 5 | 1 | 1 |
| SANITRY MAIN | RW011 | Hazel Ave between Connaught & | 1955 | 9 | 305 | CONCRETE | FALSE | 6 | CIPP 2 | \$ 6,231 | 3 | 3 | 1 |
| SANITRY MAIN | SH032 | Alley Between Willow and Pine | 1955 | 30 | 203 | VCT | TRUE | 1 | PIPE BURST 2 | \$ 22,800 | 5 | 1 | 3 |
| SANITARY MH | 36B4 | 895 Pyramid Lake Road | 1973 | 2 | 1200 | UNKNOWN | TRUE | 5 | SPIN CAST | \$ 4,001 | 4 | 1 | 1 |

With respect to storm water pipes, manholes, and catch basins, there were no major works generated for any of these infrastructure groups. Overall, the storm water collection system is in a better condition state than the sanitary collection system.

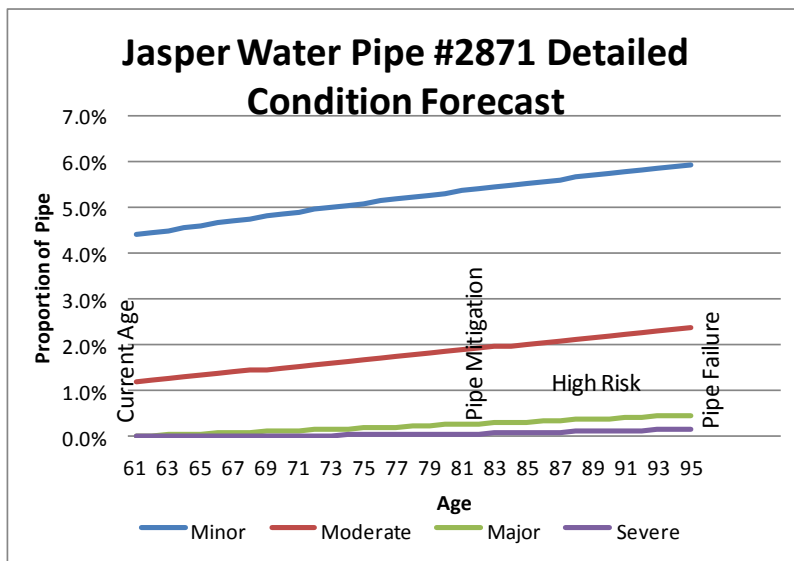
4.4 Water Distribution (Pipes and Valves)

The water distribution comprises of two main components. One is the pipe. The other is associated valves, including isolation valves, hydrants, and service connection taps.

Collecting the condition assessment on the water distribution system is difficult due to access availability to the inside of a pressure pipe. The condition assessment procedure used electromagnetic technology for determining the severity and extent of pipe remaining wall thickness. The only pipe the condition assessment contractor was able to access was water main #2871 located in Patricia Circle. It is a 61 year old cast-iron 150 mm (6”) pressure pipe water main. Overall, the pipe is in relatively good condition. It has approximately 5-6% of the surface area pitted. The majority is in the minor severity grouping (i.e. 50-80% remaining wall thickness). There is no major (5-20% remaining wall thickness) or severe (0-5% remaining wall thickness) pitting (failure). Based on the age, material type, and condition measurements attained, this pipe was used to correlate a condition assessment for the remainder of the water main piping network.

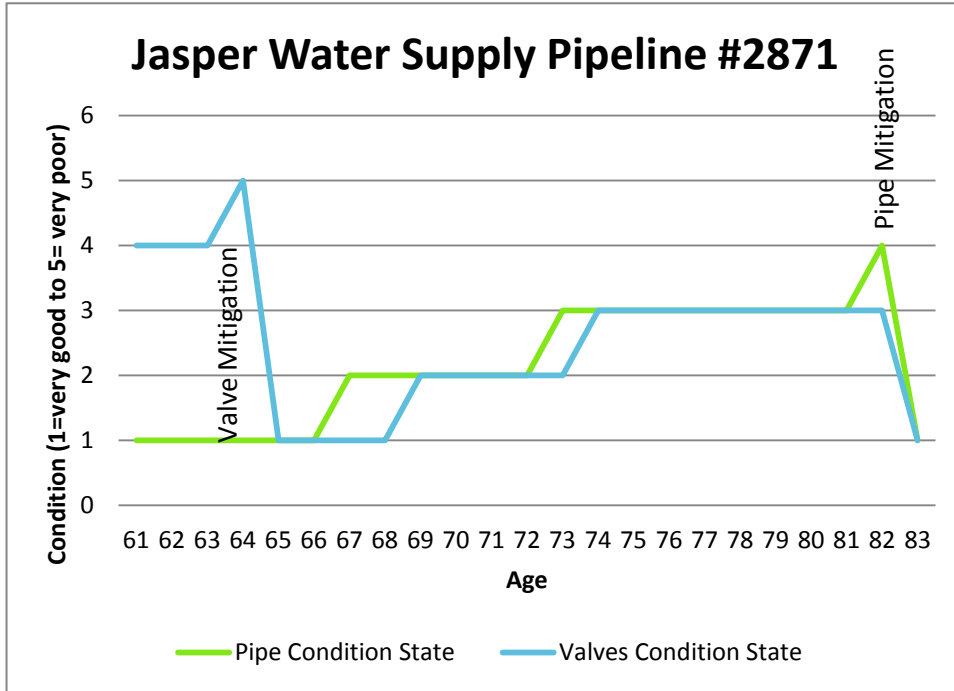
Based on the piping assessment, it was observed that the valves were deteriorating at a different rate than the pipe. As example, with pipe #2871, the following figures shows the modelled deterioration estimating a remaining service life of 20-30 years, while know valve failures associated in this are now reaching the end of their service life.

Figure 28 – Water Main #2871



The following figure shows the expectation of renewal needs between pipe and valve. In the prediction model, we include a probabilistic allowance for valve failure and associated valve repair within the pipe lifecycle. Then when the pipe is eventually replaced, renewal occurs for all components associated with the pipe (i.e. pipe, isolation valves, hydrants, and service connection).

Figure 29– Condition State Performance Prediction - Water Main #2871



The following figure shows a valve repair that was occurring concurrent with the data collection within pipe segment #2871 in Patricia Circle.

Figure 30– Patricia Circle Water Main Pipe #2871 Valve Assembly Replacement (Year 2016)



The following figures work together in illustrating the expenditure needs versus performance of the water distribution infrastructure assets. The performance prediction models are indicating that the overall state of the water pipelines will remain in a very good “1” condition state for the next 14 years. At this time, there is expected to be approximately \$9 Million in pipeline renewal activity required. The associated valves are currently in a fair “3” condition state with on-going repair/replacement needed over the next 22 year. In year-22, it is expected the system will sustain a good “2” condition state until year 38, when the next surge of pipeline renewal is expected again.

Figure 31– Water Mains Long-Range Performance Prediction Projection

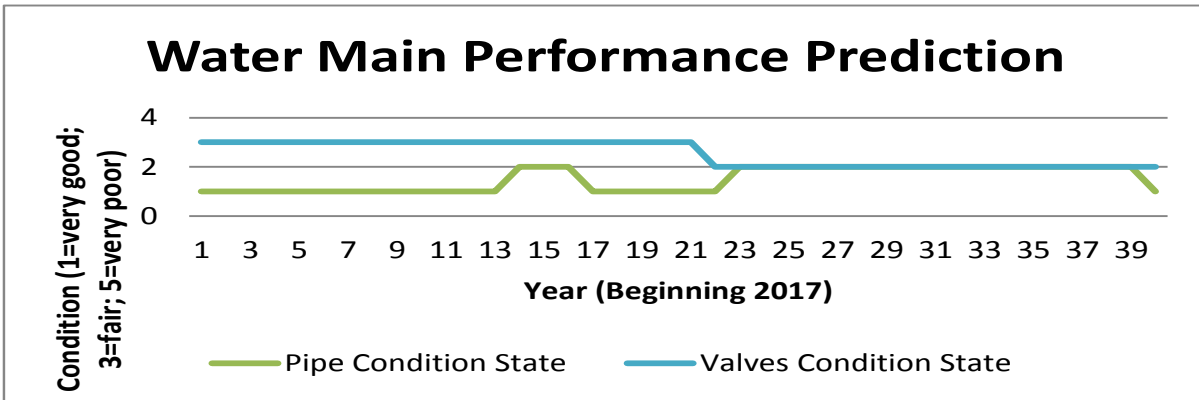
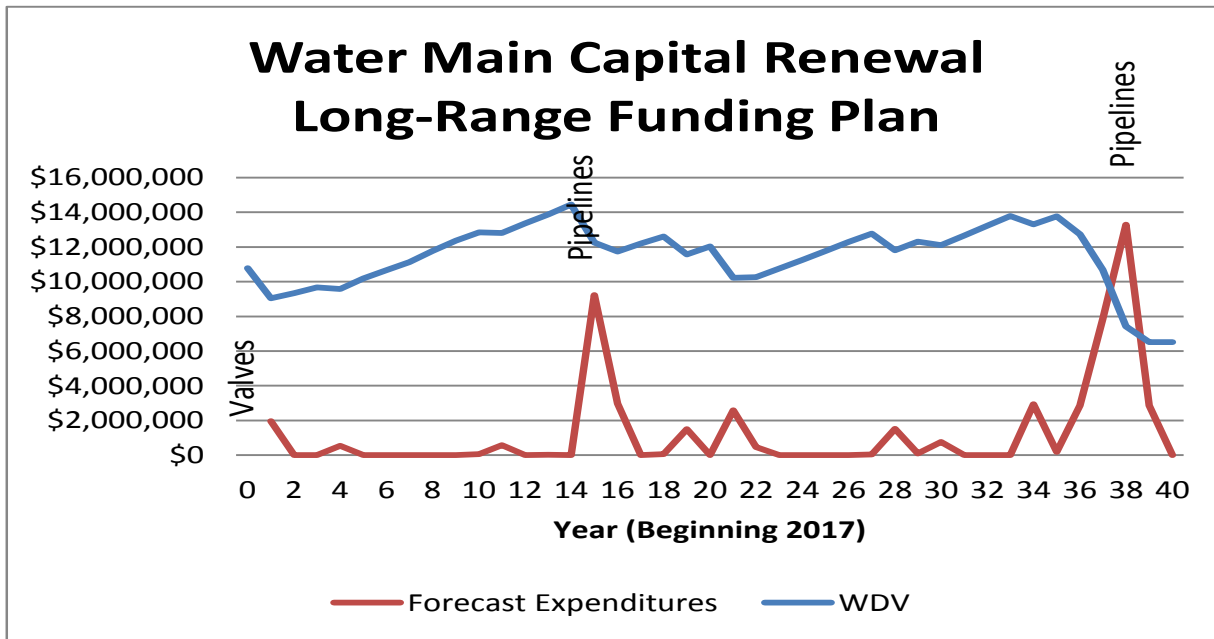


Figure 32– Water Mains Long-Range Funding Projection



As the deterioration cycles appear to be different, between the pipes and its associated valves, the short-term (i.e. 5-year) expenditure needs do not include any pipe replacement. The short-term allocation needs are entirely associated with the probability of valve failure and associated emergency repairs.

The following table measures the effectiveness of the proposed spending in the short-term (i.e. 5-year) and long-term (40-year) horizons for the sanitary mains. The results are indicating a 1%/yr short-term and 2%/yr long-term improvement to the condition of the water mains. This was measured by the physical condition index along with the monetary WDV. Both are relatively consistent. The ROII, which indicates the asset improvement above sustainability level, is modest but positive. The short-term is specific to valve repair. The long-term related to pipeline renewal, including replacement of valves and pipes together.

Table 18: – Water Mains Performance Summary

| Water Mains | Optimization 5-Year | Optimization 40-Year |
|-----------------------------------|--------------------------------|---------------------------------|
| Program Expenditures | | |
| - Total Period Expenditure (M\$) | \$2.500 | \$52.375 |
| - Annualized Expenditure (M\$/yr) | \$0.500 | \$1.309 |
| Monetary Performance | | |
| - WDV (initial) | \$9.912 | \$9.912 |
| - WDV (Final) | <u>\$9.328</u> | <u>\$2.538</u> |
| Improved Perf (+) | \$0.584 | \$7.374 |
| Annual Perf Change (M\$/yr) | \$0.117 | \$0.184 |
| | 1% | 2% |
| Condition State/Index | | |
| - Index (initial) | 17 | 17 |
| - Index (Final) | <u>16</u> | <u>4</u> |
| Improved Condition (+) | 1 | 13 |
| Annual Condition Change (/yr) | 0.2 | 0.3 |
| Annual Condition Change (%/yr) | 1% | 2% |
| ROII - Annualized (%/yr) | 123% | 114% |
| - (100% = Stabilization Level) | | |

The following summarizes where the expected short-term expenditures may be allocated. We are not showing an immediate need for water main capital renewal. However, we are showing a relatively significant allocation to associated valve repair needs required to sustain the pipeline until the pipeline renewal period begins in approximately 14 years from now.

Table 19– Water Mains Five-Year (2017-2021) Maintenance and Renewal Summary

| Treatment Activity | Length (m) | Cost (\$/yr) |
|---------------------------------------|-------------------|---------------------|
| Valve Repair (inc. other pipe repair) | | \$500,000 |
| Pipe Bursting (i.e. pipe replacement) | | \$0 |
| Average Annual Cost | | \$500,000 |
| | | |
| Total Cost Over 5 Years | | \$2,500,000 |

The following tables list the locations of the high risk areas that may expect valve failure in the short-term horizon. The Municipality may be pragmatic in anticipating failures within these locations and others. If failures are observed to repeat within the pipe segment, the Municipality may wish to consider scheduled valve replacement of the remaining valves and connections along these pipe segments. These tables also list the projected time of pipe replacement. The time gap between valve failure and full line replacement is approximately 14 years. It may be desirable to bridge the time gap by monitoring and repairing unless a significant amount of repairs are noted on any pipe segment as logged within the Municipality's maintenance management system.

Table 20 – Five-Year (2017-2021) High Risk Valve Failure Locations

| ITEM | SEGMENT | AGE | LENGTH | DIA | MATERIAL | Year | | COST | Pipe | Valve | Capacity | Year |
|------------|---------|------|--------|-----|-----------|-------|------------|------------|------|-------|----------|------|
| | | | | | | Valve | TREATMENT | | | | | |
| WATER MAIN | 1DA7 | 1950 | 17 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 6,720 | 2 | 5 | 1 | 15 |
| WATER MAIN | 1DA9 | 1950 | 16 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 6,300 | 2 | 5 | 1 | 15 |
| WATER MAIN | 1DAB | 1950 | 21 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 2,940 | 2 | 5 | 4 | 15 |
| WATER MAIN | 1DAC | 1950 | 5 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 840 | 2 | 5 | 4 | 15 |
| WATER MAIN | 1DB0 | 1955 | 6 | 50 | CAST IRON | 4 | VALVE FAIL | \$ 794 | 2 | 5 | 4 | 17 |
| WATER MAIN | 273E | 1950 | 7 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 2,940 | 2 | 5 | 1 | 15 |
| WATER MAIN | 27E7 | 1950 | 4 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,680 | 2 | 5 | 1 | 15 |
| WATER MAIN | 27FF | 1950 | 4 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,680 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2853 | 1950 | 109 | 250 | CAST IRON | 1 | VALVE FAIL | \$ 71,820 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2854 | 1950 | 15 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 2,100 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2856 | 1950 | 7 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 2,940 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2859 | 1950 | 5 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 2,100 | 2 | 5 | 1 | 15 |
| WATER MAIN | 285A | 1950 | 10 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 3,780 | 2 | 5 | 1 | 15 |
| WATER MAIN | 285B | 1950 | 32 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 12,600 | 2 | 5 | 1 | 15 |
| WATER MAIN | 285E | 1955 | 46 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 17,073 | 2 | 5 | 1 | 17 |
| WATER MAIN | 285F | 1955 | 19 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 7,147 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2860 | 1955 | 15 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 5,559 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2861 | 1955 | 8 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 3,176 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2862 | 1955 | 24 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 9,132 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2863 | 1955 | 12 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 4,368 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2864 | 1955 | 34 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 12,706 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2869 | 1955 | 22 | 250 | CAST IRON | 4 | VALVE FAIL | \$ 13,897 | 2 | 5 | 1 | 17 |
| WATER MAIN | 286A | 1955 | 118 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 44,073 | 2 | 5 | 1 | 17 |
| WATER MAIN | 286B | 1955 | 6 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 2,382 | 2 | 5 | 1 | 17 |
| WATER MAIN | 286C | 1955 | 11 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 3,971 | 2 | 5 | 1 | 17 |
| WATER MAIN | 286F | 1955 | 11 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 3,971 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2871 | 1955 | 115 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 42,882 | 1 | 5 | 1 | 22 |
| WATER MAIN | 2872 | 1955 | 13 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 4,765 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2874 | 1955 | 37 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 13,897 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2875 | 1955 | 21 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 7,941 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2879 | 1950 | 181 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 71,400 | 2 | 5 | 1 | 15 |
| WATER MAIN | 287A | 1950 | 4 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,680 | 2 | 5 | 1 | 15 |
| WATER MAIN | 287B | 1950 | 45 | 200 | CAST IRON | 1 | VALVE FAIL | \$ 23,940 | 2 | 5 | 1 | 15 |
| WATER MAIN | 287E | 1950 | 37 | 200 | CAST IRON | 1 | VALVE FAIL | \$ 19,740 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2882 | 1950 | 9 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 3,360 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2883 | 1950 | 174 | 250 | CAST IRON | 1 | VALVE FAIL | \$ 114,660 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2884 | 1950 | 19 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 2,520 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2886 | 1950 | 47 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 6,300 | 2 | 5 | 4 | 15 |

Table 21 – Five-Year (2017-2021) High Risk Valve Failure Locations

| ITEM | SEGMENT | AGE | LENGTH | DIA | MATERIAL | Year | | COST | Pipe | Valve | Capacity | Year |
|------------|---------|------|--------|-----|--------------|-------|------------|-----------|------|-------|----------|------|
| | | | | | | Valve | TREATMENT | | | | | |
| WATER MAIN | 2887 | 1950 | 3 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 420 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2888 | 1950 | 16 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 2,100 | 2 | 5 | 4 | 15 |
| WATER MAIN | 288C | 1950 | 142 | 150 | PVC | 1 | VALVE FAIL | \$ 55,860 | 2 | 5 | 1 | |
| WATER MAIN | 288D | 1950 | 23 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 9,240 | 2 | 5 | 1 | 15 |
| WATER MAIN | 288E | 1950 | 17 | 150 | ASBES CEMENT | 1 | VALVE FAIL | \$ 6,720 | 2 | 5 | 1 | 7 |
| WATER MAIN | 2891 | 1950 | 60 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 23,520 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2893 | 1950 | 8 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 840 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2894 | 1950 | 11 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 1,260 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2897 | 1950 | 15 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 2,100 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2898 | 1950 | 39 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 5,040 | 2 | 5 | 4 | 15 |
| WATER MAIN | 289C | 1950 | 142 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 55,860 | 2 | 5 | 1 | 15 |
| WATER MAIN | 289D | 1950 | 20 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 7,980 | 2 | 5 | 1 | 15 |
| WATER MAIN | 289E | 1950 | 50 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 19,740 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28A1 | 1950 | 169 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 66,780 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28A3 | 1950 | 41 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 16,380 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28A4 | 1950 | 40 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 15,960 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28BD | 1950 | 193 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 76,020 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28BE | 1950 | 3 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,260 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28C0 | 1950 | 46 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 18,060 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28C2 | 1950 | 3 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,260 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28C3 | 1950 | 9 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 1,260 | 2 | 5 | 4 | 15 |
| WATER MAIN | 28C4 | 1950 | 33 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 8,820 | 2 | 5 | 4 | 15 |
| WATER MAIN | 28C8 | 1950 | 45 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 11,760 | 2 | 5 | 4 | 15 |
| WATER MAIN | 28CB | 1950 | 95 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 37,380 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28CC | 1950 | 72 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 28,560 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28CD | 1950 | 3 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 840 | 2 | 5 | 4 | 15 |
| WATER MAIN | 28CE | 1950 | 18 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 7,140 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28CF | 1950 | 59 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 23,100 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28D0 | 1950 | 40 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 15,960 | 2 | 5 | 1 | 15 |
| WATER MAIN | 28D2 | 1950 | 65 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 17,220 | 2 | 5 | 4 | 15 |
| WATER MAIN | 293A | 1950 | 31 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 12,180 | 2 | 5 | 1 | 15 |
| WATER MAIN | 293B | 1950 | 4 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,680 | 2 | 5 | 1 | 15 |
| WATER MAIN | 293C | 1950 | 31 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 12,180 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2940 | 1950 | 61 | 200 | CAST IRON | 1 | VALVE FAIL | \$ 32,340 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2941 | 1950 | 99 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 39,060 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2947 | 1950 | 3 | 200 | CAST IRON | 1 | VALVE FAIL | \$ 1,680 | 2 | 5 | 1 | 15 |
| WATER MAIN | 294C | 1950 | 48 | 250 | CAST IRON | 1 | VALVE FAIL | \$ 31,500 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2955 | 1950 | 26 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 10,080 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2956 | 1950 | 105 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 41,580 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2957 | 1950 | 41 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 16,380 | 2 | 5 | 1 | 15 |
| WATER MAIN | 298C | 1955 | 206 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 77,028 | 2 | 5 | 1 | 17 |
| WATER MAIN | 29A7 | 1955 | 189 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 70,675 | 2 | 5 | 1 | 17 |
| WATER MAIN | 29AC | 1955 | 204 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 76,234 | 2 | 5 | 1 | 17 |
| WATER MAIN | 29CE | 1955 | 18 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 6,750 | 2 | 5 | 1 | 17 |
| WATER MAIN | 29CF | 1955 | 87 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 32,558 | 2 | 5 | 1 | 17 |
| WATER MAIN | 29D2 | 1955 | 61 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 22,632 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2B14 | 1955 | 4 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 1,588 | 2 | 5 | 1 | 17 |

Table 22 – Five-Year (2017-2021) High Risk Valve Failure Locations

| ITEM | SEGMENT | AGE | LENGTH | DIA | MATERIAL | Year | | | Pipe | Valve | Capacity | Year |
|------------|---------|------|--------|-----|-----------|-------|------------|------------|------|-------|----------|------|
| | | | | | | Valve | TREATMENT | COST | | | | |
| WATER MAIN | 2C54 | 1950 | 9 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 3,360 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C55 | 1950 | 166 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 65,520 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C60 | 1955 | 35 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 13,103 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2C61 | 1955 | 59 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 21,838 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2C62 | 1955 | 34 | 150 | CAST IRON | 4 | VALVE FAIL | \$ 12,706 | 2 | 5 | 1 | 17 |
| WATER MAIN | 2C63 | 1950 | 13 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 5,040 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C64 | 1950 | 156 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 61,740 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C66 | 1950 | 31 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 8,400 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C67 | 1950 | 4 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 1,260 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C69 | 1950 | 54 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 14,280 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C6A | 1950 | 71 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 18,900 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C6B | 1950 | 59 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 15,540 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C6C | 1950 | 30 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 3,780 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C6D | 1950 | 62 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 7,980 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C6E | 1950 | 5 | 50 | CAST IRON | 1 | VALVE FAIL | \$ 840 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C6F | 1950 | 69 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 18,060 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C70 | 1950 | 47 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 12,600 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C71 | 1950 | 50 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 13,440 | 2 | 5 | 4 | 15 |
| WATER MAIN | 2C74 | 1950 | 51 | 200 | CAST IRON | 1 | VALVE FAIL | \$ 26,880 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C75 | 1950 | 8 | 200 | CAST IRON | 1 | VALVE FAIL | \$ 4,200 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C87 | 1950 | 4 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,680 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C8E | 1950 | 3 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,260 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C8F | 1950 | 25 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 10,080 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C90 | 1950 | 58 | 200 | CAST IRON | 1 | VALVE FAIL | \$ 30,660 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C92 | 1950 | 3 | 200 | CAST IRON | 1 | VALVE FAIL | \$ 1,680 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C94 | 1950 | 4 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,680 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C95 | 1950 | 3 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,260 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2C96 | 1950 | 4 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 1,680 | 2 | 5 | 1 | 15 |
| WATER MAIN | 2CBD | 1950 | 5 | 150 | PVC | 1 | VALVE FAIL | \$ 2,100 | 2 | 5 | 1 | |
| WATER MAIN | 2CBE | 1950 | 5 | 150 | PVC | 1 | VALVE FAIL | \$ 2,100 | 2 | 5 | 1 | |
| WATER MAIN | 2CC2 | 1950 | 11 | 150 | PVC | 1 | VALVE FAIL | \$ 4,200 | 2 | 5 | 1 | |
| WATER MAIN | 2CC3 | 1950 | 26 | 150 | PVC | 1 | VALVE FAIL | \$ 10,080 | 2 | 5 | 1 | |
| WATER MAIN | 2CC4 | 1950 | 7 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 2,940 | 2 | 5 | 1 | 15 |
| WATER MAIN | 3EE1 | 1950 | 53 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 13,860 | 2 | 5 | 4 | 15 |
| WATER MAIN | 3EF6 | 1950 | 235 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 92,820 | 2 | 5 | 1 | 15 |
| WATER MAIN | 3EF8 | 1950 | 316 | 100 | CAST IRON | 1 | VALVE FAIL | \$ 83,580 | 2 | 5 | 4 | 15 |
| WATER MAIN | 3EF9 | 1950 | 182 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 71,820 | 2 | 5 | 1 | 15 |
| WATER MAIN | 42EF | 1950 | 110 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 43,260 | 2 | 5 | 1 | 15 |
| WATER MAIN | 42F0 | 1950 | 44 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 17,220 | 2 | 5 | 1 | 15 |
| WATER MAIN | 42F1 | 1950 | 30 | 250 | CAST IRON | 1 | VALVE FAIL | \$ 19,740 | 2 | 5 | 1 | 15 |
| WATER MAIN | 42F2 | 1950 | 6 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 2,520 | 2 | 5 | 1 | 15 |
| WATER MAIN | 44AA | 1950 | 23 | 50 | PVC | 1 | VALVE FAIL | \$ 2,940 | 2 | 5 | 4 | |
| WATER MAIN | 44C6 | 1950 | 199 | 250 | CAST IRON | 1 | VALVE FAIL | \$ 131,040 | 2 | 5 | 1 | 15 |
| WATER MAIN | 44E0 | 1950 | 6 | 200 | CAST IRON | 1 | VALVE FAIL | \$ 3,360 | 2 | 5 | 1 | 15 |
| WATER MAIN | 4517 | 1950 | 11 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 4,200 | 2 | 5 | 1 | 15 |
| WATER MAIN | 4568 | 1950 | 72 | 150 | CAST IRON | 1 | VALVE FAIL | \$ 28,560 | 2 | 5 | 1 | 15 |

5. Program Summary

The following table summarizes the short-term (5-year) and long-term infrastructure renewal needs. The summary from the tactical level is compiled from above and deemed to be the most accurate due to the field level of condition rating assessments and associated analysis for each infrastructure segment in the network. This is compared to the strategic level long-range funding plan completed in December 2015.

Table 23 – Short-Range (2017-2021) and Long-Range Funding Plan Summary

| Asset Group | Tactical | | | | Reserve Fund Annual Accumulation (+) building; (-) drawing (M\$/yr) | Strategic Long-Term Sustainability Needs Projection (M\$/yr) | Historic Current Budget Allocation (M\$/yr) | Difference Sustainability to Budget Surplus (+); Deficit (-) (M\$/yr) |
|----------------|---|---|--|----------------|---|---|---|--|
| | Short-Term (5 Year) Needs Total (M\$) | Short-Term (5 Year) Needs Annual (M\$/yr) | Long-Term Sustainability Needs Annual (M\$/yr) | | | | | |
| Roadways | | | | | | | | |
| - Streets | \$2.660 | \$0.532 | \$0.230 | | | | | |
| - Sidewalks | <u>\$0.375</u> | <u>\$0.075</u> | <u>\$0.074</u> | | | | | |
| Sub-Total | \$3.035 | \$0.607 | \$0.304 | -\$0.303 | \$1.220 | \$0.710 | \$0.406 | |
| Wastewater | | | | | | | | |
| - Collection | \$0.636 | \$0.127 | \$0.201 | | | | | |
| - Treatment | TBD | TBD | <u>\$0.415</u> | | | | | |
| Sub-Total | \$0.636 | \$0.127 | \$0.616 | \$0.489 | \$0.730 | \$0.130 | -\$0.486 | |
| Water | | | | | | | | |
| - Distribution | \$2.500 | \$0.500 | \$1.309 | | | | | |
| - Treatment | TBD | TBD | <u>\$0.332</u> | | | | | |
| Sub-Total | <u>\$2.500</u> | <u>\$0.500</u> | <u>\$1.641</u> | <u>\$1.141</u> | <u>\$0.940</u> | <u>\$0.310</u> | <u>-\$1.331</u> | |
| Total | \$6.171 | \$1.234 | \$2.561 | \$1.327 | \$2.890 | \$1.150 | -\$1.411 | |

In reference to the above table, tactical level assessment of the treatment facilities (incl. lift stations) for the water and wastewater asset groups was not yet completed. Such will be completed following this report. The short term-needs is not yet know for the treatment facilities. However, the strategic level report provided a reasonable understanding of the long-term expenditure needs. This was included above. There is reasonable chance the short-term capital renewal expenditure needs for treatment facilities may be slight due to the relatively new age of these facilities. The treatment facilities short-term funding needs are marked as "To Be Determined" for the interim, until that component if the infrastructure assessment is complete. However, due to the expectation of minimal expenditure needs of the treatment facilities in the short-term, the above table is relatively complete for preliminary discussions on budget allocations and capital renewal programming.

For the asset groups of Roadways, Wastewater, and Water, the short-term capital renewal expenditure needs is collectively \$1.234 Million per year. This is what the Municipality should be spending over the next five years as per the details provided in this report. However, what the Municipality should be allocating in its capital programming is \$2.561 Million per year to provide a sustainable funding level. The surplus of \$1.327 Million per year may be allocated to a Reserve Fund for use beyond year 2021 when there will be significantly greater spikes in capital renewal expenditure needs.

6. Conclusions and Recommendations

The following summarizes key conclusions and recommendation with respect to the given Municipality of Jasper Tactical Level Asset Management Study.

6.1 Conclusions

- The roadways (streets and sidewalks) infrastructure group has the greatest need for infrastructure renewal. However, the expenditure needs are less than previously estimated due to state of the infrastructure that can still permit cost effective mitigation through “Micro-Surfacing” that can renew the roadways infrastructure at approximately ¼ the cost of conventional resurfacing or replacement methods. The five-year horizon has targeted \$1.7 Million towards streets micro-surfacing. Timeliness on implementation is important. If such works are deferred, the streets will undergo further deterioration in which the renewal costs could escalate to \$6.8 Million if conventional pavement resurfacing becomes required.
- The wastewater collection (sanitary and storm) infrastructure group is in a condition state where a relatively small component of the sanitary piping system is in need of renewal today. The majority of these works may be completed through a relatively cost effective cured in place pipe (CIPP) trenchless technology (i.e. approximately ¾ the cost of pipeline replacement). Overall, the long-range sustainable funding levels are close to previously anticipated. Initiating the CIPP renewal practice on a small proportion of the network today will get the practice in place later in the long-term when more renewal will be required.
- The water distribution infrastructure group has a cast iron piping network that is in better condition than previously estimated. However, this is based on analysis of one pipe that was accessible for a condition assessment. The performance prediction analysis is forecasting a significant amount pipeline replacement needs in approximately 14 years from today. When this occurs, it would be replacement of the pipeline and associate valve components (i.e. isolation valves, hydrants, and service connections). Until then, the valve related components may fail earlier and will have to be replaced on an emergency basis as the failure occurs; in which the funding allocation needs appear to be manageable. The short-range high-risk valve failure pipe segments are listed in this report. This gives the opportunity for the Municipality to develop a Maintenance Management System (MMS) and begin logging pipe or valve failures. On high risk pipe segments with repeat failures, the Municipality may consider the options to:
 - i. Continue with emergency valve/pipe repair until the pipe is scheduled for replacement, or
 - ii. Conduct a scheduled valve replacement as an interim measure until the pipe segment replaced, or
 - iii. Accelerate the pipe replacement, including associated valves, hydrants and fittings.
- The GIS contains information gaps. During the condition assessment, sections of the Town’s sidewalks were missing. The GIS inventory states that water distribution pipelines are of a cast iron material type. There is potential, the metallic pipes installed in the 1980s could be ductile iron, which has a lower service life than cast iron. Pipeline diameters measured during emergency repairs were sometimes different to what was recorded in the GIS inventory. A full infrastructure inventory (GIS) review of the roadways (streets and sidewalks), wastewater collection (storm water and sanitary sewer piping, manholes, catch basins, service connections), and water distribution (pipes, valves, hydrants, service connections) could update the GIS through combined efforts of record drawing reviews and field investigation. The field investigation completed in part to investigating the subsurface infrastructure upon completion of emergency repairs.
- The extent of physical condition inspection of the various infrastructure assets was dependent on access. The surface assets (i.e. streets and sidewalks) were easily accessed for assessment. Wastewater collection was more difficult, requiring access through manholes. Water distribution was the most difficult, requiring access through fire hydrants. For infrastructure assets not physically assets, their corresponding condition assessment was based on correlation from findings of the infrastructure assets that were assessed. The following summarizes the proportion of the infrastructure assets for which the condition assessment was

based on a physical inspection or a correlation. Condition inspections need to be repeated on a regular basis (i.e. 2-4 years). However, the order of priority should be for the water distribution system followed by the wastewater collection (storm and sanitary). Access to the water distribution system may be based on opportunity. Times of planned excavation and repair to the water pipes and valves, where the pipe is opened, may be used to schedule condition assessment of the pipe segment as a whole. Continuous efforts on condition assessments and subsequent analysis will improve reliability in asset management.

Table 24 – Condition Assessment Basis

| Asset Group | Physically Inspected | Correlated |
|--------------------------|-----------------------------|-------------------|
| Roadways (Streets) | 100% | 0 |
| Roadways (Sidewalks) | 100% | 0 |
| Sanitary Mains | 26% | 74% |
| Storm Water Mains | 10% | 90% |
| Water Distribution Mains | 0.3% | 99.7% |

- Overall, the short-term capital renewal expenditure needs of \$1.234 Million/year for the next five years are in line with existing budget levels of \$1.150 Million/year. However, to meet the long-term sustainability needs, the current-day capital renewal budget needs are \$2.561 Million per year.
- This study provides details what is the recommended treatment type, where to do the treatment and, when to do the treatment. Even though these detailed works programs are spread out over five years, practicality may combine works of a similar nature in a contract deployed in one of the five years. In all cases, a review of the infrastructures should be completed as a check before progressing to design and tender; in particular to any changes that may have occurred since the inspections.
- Even though the time-frame for this study goes decades into the future, the critical components are within the short-term (five-year) horizon. To sustain capital renewal programming reliability, a tactical level asset management plan should be repeated on 2-4 year cycles. Each analysis cycle would reproduce the 5-year capital renewal program and strategy.
- As the above works programs unfold, including maintenance and repairs, this work, including type of activity, cost, data, and location (including segment identification) be captured in the form of a suitable maintenance management system (i.e. database application); and this application contain link to the existing GIS.

6.2 Recommendations

- i. That the Municipality of Jasper implements the short-term (five-year) capital renewal program as per the tactical level analysis and results attained within this report and to the following allocations; annualized at \$1.234 Million/year.

Table 25 – Short-Range (5-year) Capital Renewal Recommended Funding Allocations

| Asset Group | Short-Term (5 Year) Needs Total (M\$) |
|--------------------|--|
| Roadways | |
| - Streets | \$2.660 |
| - Sidewalks | <u>\$0.375</u> |
| Sub-Total | \$3.035 |
| Wastewater | |
| - Collection | \$0.636 |
| - Treatment | <u>TBD</u> |
| Sub-Total | \$0.636 |
| Water | |
| - Distribution | \$2.500 |
| - Treatment | <u>TBD</u> |
| Sub-Total | <u>\$2.500</u> |
| Total | \$6.171 |

- ii. That the Municipality recognize the long term sustainability funding level of \$2.561 Million per year for the roadways, water, and wastewater (storm water and sanitary) infrastructure groups.
- iii. That the sustainable funding level is used in applying for external funding grants and consideration in tax and utility rates moving forward.
- iv. That the Municipality implements an infrastructure inventory (GIS) review to update infrastructure records.
- v. That the Municipality develops a Maintenance Management System (MMS) to track maintenance and renewal works; and that this is a database application with link to the existing GIS.
- vi. That the Municipality repeat this tactical level asset management analysis and study on 2-4 year cycles; with a focus on assessing more water distribution pipes where opportune to do so.

Appendix A
Condition Assessment Criteria

Municipality of Jasper

Asset Management Condition Rating Criteria

- Final

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Project Number:

Phase 2

Date:

2017-03-10

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1. Introduction

The Municipality of Jasper Condition Rating Criteria is designed to provide consistency and structure in periodic condition assessments of the various asset groups. This document will be used as the basis for field condition rating assessments beginning the summer of 2016.

The criterion for each asset group is unique. However, the common element for all condition assessments is the “Severity” and “Extent” format. Severity measures the extremity of the condition rating. This would typically be None, Minor, Moderate, Major, and Severe. Extent measures the proportion of the asset segment within each of these severity categories.

The Severity-Extent condition data format is the foundation of the lifecycle optimization model. The model uses this information in both performance predictions over time as well as infrastructure renewal and treatment selection.

It is expected the assessment criteria may evolve over time. However, there is advantage in maintaining consistency of the given condition rating framework. The first year of two using the given condition rating criteria may establish an infrastructure performance benchmark. Delivery of the recommended program into the future may be used later to compare the state of the infrastructure at that time to the 2016 condition assessment benchmark.

2. Roadways – Pavements

The roadways condition assessment is based on the following four distresses (i.e. rutting, lineal cracking, fatigue cracking, and surface condition) and one geometric condition (i.e. grade). An experienced rater will be able to complete the assessment based on a visual review of each segment. A less experienced rater would be expected to conduct measurements of sample areas within the roadway segment.

2.1 Rutting

Definition

Rutting is the longitudinal surface depression developing in the wheel paths due to repeated load applications. One wheel path depression may have single or double ruts.

Severity

- None - Rut depths less than 5 mm
- Minor (slight) - Rut depths of 5 -10 mm
- Moderate - Rut depths of 11 to 15 mm
- Major (Extreme) - Rut depths greater than 15 mm

Extent

The extent will be determined as the percentage of visual measurements in each severity category. The extent will be of the proportion of rutted area as a ratio of the entire road surface.



2.2 Lineal Cracking

Definition

Lineal Cracks are single line cracks that have not formed into blocks, either transverse or longitudinal.

A nominal width of 0.1 metres is assigned to that crack. A nominal area can then be calculated (0.1m X length of crack). Any adjacent crack less than 1 m apart should be included together and considered under Fatigue Cracking.

Severity

- None – No cracking area
- Minor - Crack width < 12.5mm
- Moderate - crack width between 12.5mm and 25mm
- Major - crack width > 25 mm

Extent

The extent is the percentage of each severity area to the road surface area.

Repaired areas will be assessed only on cracking that has reflected through the repair.



2.3 Fatigue Cracking

Definition

Cracks that form a block, or adjacent cracks that are less than 1 m apart.

The area of a fatigue block is the length multiplied by the width.

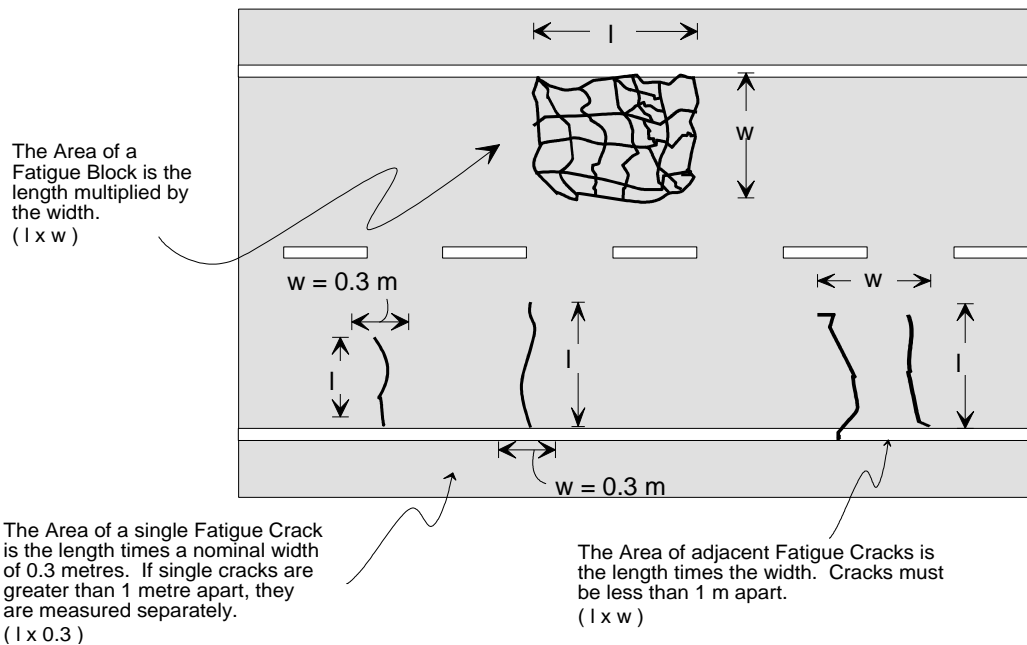
Severity

- None – No cracking area
- Minor (slight) - Short side of block having a length between 0.4 m and 1.0 m
- Moderate - Short side of block having a length less than 0.4 m
- Major (extreme) - Short side of block being less than 0.4 m and block area loose or picking out

NOTE: Any side of a block is not to exceed 1.0 metres. If a block exceeds 1.0 metres, each crack compromising the block is reported as a lineal crack (i.e. length of crack x 0.1m).

Extent

The area of cracking is reported for each severity level. The extent is the percentage of each severity area to the road surface area.



2.4 Surface Condition

Definition

Surface condition is an assessment of the pavement surface with respect to raveling, segregation and loss of aggregate.

Raveling is the progressive separation of aggregate particles in a pavement from the surface. Weathering, construction techniques, construction materials and the abrasive action of traffic can all cause raveling. Usually, the fine aggregates separate first and leave little "pock marks" on the pavement surface. As the separation continues, larger and larger particles will break free leaving the pavement with a rough and jagged appearance, including potholes.

Segregation is the separation of the coarse aggregate particles from the finer particles in a new asphalt mix.

Loss of aggregate in pavements occurs when single aggregates are removed from the surface creating a "pick-out". Eventually, the pick-out will grow as the asphalt concrete mix is worn away from the hole. This defect is usually evenly distributed throughout the pavement surface. Soft or fractured aggregate and lumps of silt or clay can create a pick-out.

Severity

- None
 - No noticeable deterioration
- Minor (slight)
 - Loss of pavement matrix material
 - Pavement surface may be oxidized or grey
- Moderate
 - Having an open textured appearance
 - Significant exposed aggregate
 - Road surface beginning to become rough
 - Fairly well spaced between pock marks
 - Grey or oxidized pavement surface
- Major (extreme)
 - Disintegrated with small potholes,

Extent

The area of surface condition is reported for each severity level. The extent is the percentage of each severity area to the road surface area.

Moderate Surface Condition



2.5 Grade

Definition

The grade is a measure of the amount of curb and gutter lost due to pavement rehabilitation overlays.

Severity

- None – Pavement at or below gutter elevation
- Minor – Pavement from 0 mm to 50 mm above gutter elevation
- Moderate – Pavement from 50 mm above gutter elevation to 50 mm below top of curb
- Major – Pavement higher than 50 mm below top of curb.

Extent

The extent is the percentage of each severity area to the road surface area.

3. Sidewalks

The sidewalks condition assessment is based on the following two distresses (i.e. Cracking and Spalling).

3.1 Cracking

Definition

Cracking is defined as either lineal or blocking in nature; even though blocking is not typical for cracking. Cracking can also be reflected on trip-edge potential.

A nominal width of 0.1 metres is assigned to that crack. A nominal area can then be calculated (0.1m X length of crack).

Severity

- None – No cracking area
- Minor – Vertical differential (i.e. trip-edge) < 10mm
- Moderate - Vertical differential (i.e. trip-edge) <= 10 mm to 25 mm
- Major - Vertical differential (i.e. trip-edge) > 25mm

Extent

The extent is the percentage of each severity area to the sidewalk surface area.



3.2 Spalling

Definition

Spalling is defined as surface deterioration due to pick-outs, pop-outs, or other surface abrasion. This is often caused by salts, direct impacts (i.e. wheel hits to the curb), and quality of the concrete material during construction.

Severity

- None
 - No visible defects noted.
- Minor
 - Loss of cement binder
 - Beginning of exposed aggregate
- Moderate
 - Distinct exposed aggregate and open surface texture
 - Surface is or becoming rough
- Major
 - Disintegrated surface with potholes or breaks

Extent

The area of surface condition is reported for each severity level. The extent is the percentage of each severity area to the sidewalk surface area.

Moderate Sidewalk Spalling



4. Water Distribution

4.1 Structural

Definition

The condition assessment will apply to the water main piping network. The piping materials assessed will be “Cast Iron” pipes only. PVC pipes will not be assessed as they are assumed to be within a relatively good condition state.

The condition assessment will be completed for the pipe. The condition assessment may be applied separate for the aggregate of the valves within the section of pipe.

The assessment will be based on information available from “Remaining Wall Thickness (RWT)” analysis and/or “Break History (BH)” analysis. This information will be correlated to the remainder of the network based on pipe material and “Remaining Service Life (RSL)”.

The break BH is based on the last five years of information available. If information is only available for the last two years, the BH analysis will be based on averaging the breaks over the two-year period.

The order of priority on the assessment information will be 1-RWT, 2-BH, and 3-RSL.

The direct results attained for the pipes assessed will be correlated to the remainder of the pipe network based on material type and remaining service life (RSL). The RSL was determined earlier from the Strategic Level Asset Management Analysis.

Severity

- None
 - RWT = 80% to 100%
 - BH = 0 breaks/yr/100m
 - RSL > 50 years
- Minor
 - RWT = 50% to 80%
 - BH = 0 to 0.5 breaks/yr/100m
 - RSL = 30 to 50 years
- Moderate
 - RWT = 20% to 50%
 - BH = 0.5 to 1.0 breaks/yr/100m
 - RSL = 10 to 30 years
- Major
 - RWT = 5% to 20%
 - BH = 1.0 to 2.0 breaks/yr/100m
 - RSL = 0 to 10 years
- Severe
 - RWT = 0% - 5% (incl. through hole)
 - BH > 2.0 breaks/yr/100m
 - RSL = < 0 years

Extent

The extent will be the proportion of the assessment within each of the above severity ratings. The RWT will be distributed across the severity ratings. If RWT is not available, the extent calculation using BH and RSL will be 100% to a single severity rating.

Minor Pitting (50-80 percent remaining wall thickness)



4.2 Capacity

Definition

Capacity will be based on Volume/Capacity (V/C) ratio as per information provided from water system peak period capacity modeling or other related information provided.

Severity

- None
 - V/C = 0% to 20%
- Minor
 - V/C = 20% to 50%
- Moderate
 - V/C = 50% to 80%
- Major
 - V/C = 80% to 100%
- Severe
 - V/C = >100%

Extent

The extent will be the proportion of the assessment within each of the above severity ratings. It will be typically be 100% for one of the capacity severity ratings.

5. Wastewater (Storm and Sanitary) Collection

The condition assessment will apply to gravity collection pipes and connecting manholes. The piping materials assessed will be of the “concrete” and “vitrified clay tile (VCT)” material types. PVC pipes will not be assessed as they are assumed to be within a relatively good condition state.

5.1 Structural and Operations & Maintenance

Definition

The assessment will be based on a partial network assessment using CCTV sewer photography and NASSCO's Pipeline Assessment and Certification Program (PACP). The direct results attained for the pipes assessed will be correlated to the remainder of the pipe network based on material type and remaining service life (RSL). The RSL was determined earlier from the Strategic Level Asset Management Analysis.

The condition assessments will be completed for two condition types (i.e. Structural and Operations & Maintenance (O&M)). The assessment will follow the standard 5-point grading system.

The assessment will be applied separate for the pipes and the connecting manholes and catch basins

Severity

- None (Grade 1) – Excellent condition with only minor defects detected. Near new condition state.
- Minor (Grade 2) – Good condition with defects have not begun to deteriorate. Greater than 20 years RSL.
- Moderate (Grade 3) – Fair condition with moderate defects that will continue to deteriorate. 10 to 20 years RSL expected.
- Major (Grade 4) – Severe defects that will become grade 5 defects within the foreseeable future. 5 to 10 years RSL expected.
- Severe (Grade 5) – Severe defects that require immediate action. 0 to 5 years RSL expected.

Extent

The extent is the proportion of readings within each of the above severity categories. For example, on 10 grade readings of 2-Grade 1, 3-Grade 3, 4-Grade 4, and 1 Grade 5, the following would be the extent recordings for each severity category:

- None – 20%
- Minor – 0%
- Moderate – 30%
- Major – 40%
- Severe – 10%

Moderate (Grade 3) Structural Defects



Severe (Grade 5) Structural Defects



Severe (Grade 5) O&M Defects



5.2 Capacity

Definition

Capacity will be based on Volume/Capacity (V/C) ratio as per information provided from wastewater system peak period capacity modeling or other related information provided. CCTV inspections may provide an indication of capacity based on high level water markings in the pipe.

Severity

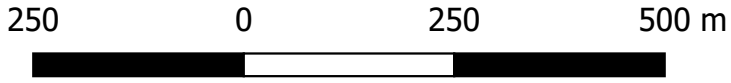
- None
 - V/C = 0% to 20%
- Minor
 - V/C = 20% to 50%
- Moderate
 - V/C = 50% to 80%
- Major
 - V/C = 80% to 100%
- Severe
 - V/C = >100%

Extent

The extent will be the proportion of the assessment within each of the above severity ratings. It will be typically be 100% for one of the capacity severity ratings.

Appendix B

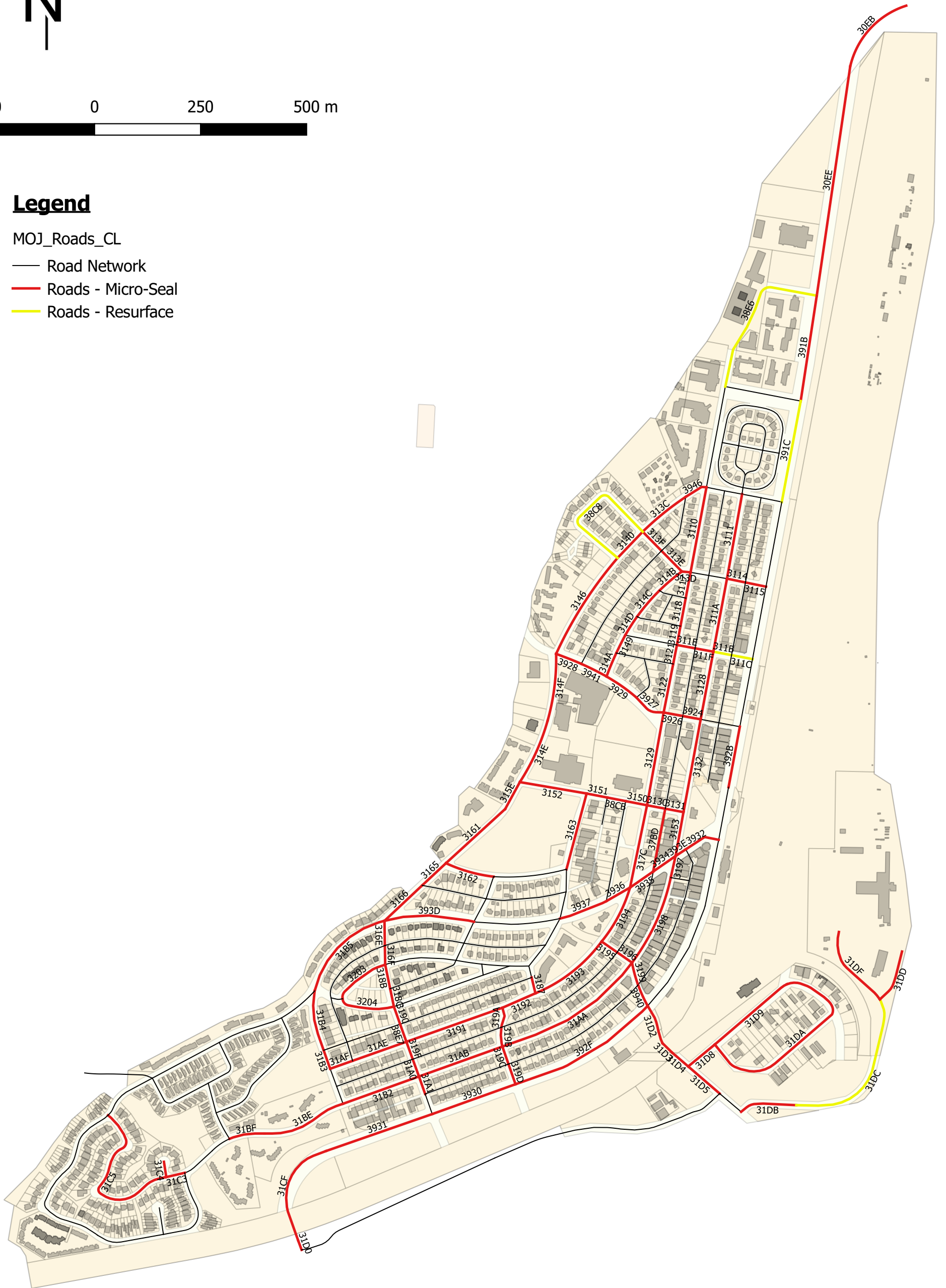
Infrastructure Life-Cycle Optimisation Modelling Runs and Maps



Legend

MOJ_Roads_CL

- Road Network
- Roads - Micro-Seal
- Roads - Resurface



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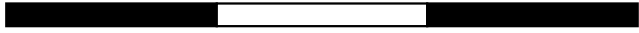
2017-04-06

Map 1 - Roadway Treatments





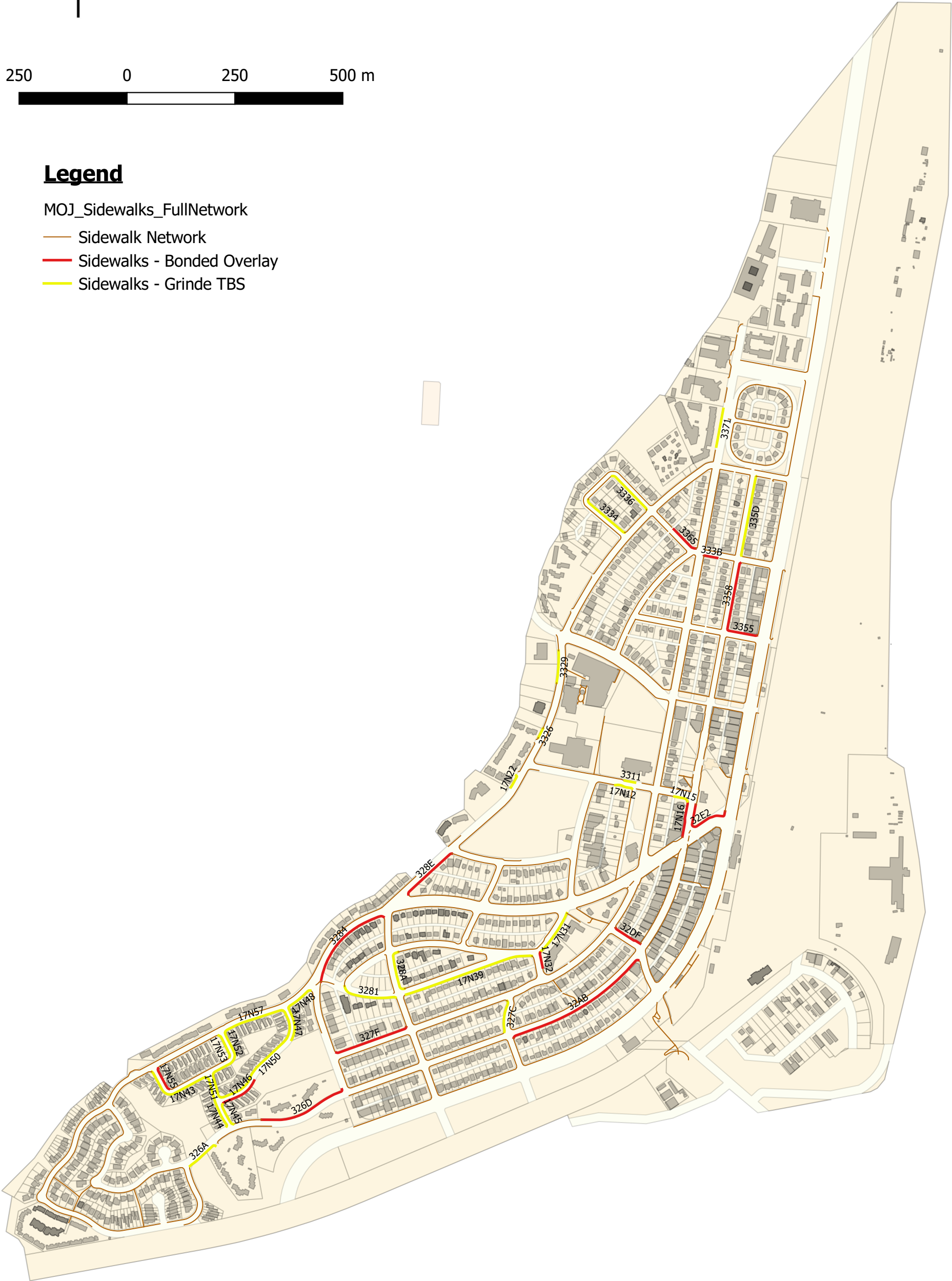
250 0 250 500 m



Legend

MOJ_Sidewalks_FullNetwork

- Sidewalk Network
- Sidewalks - Bonded Overlay
- Sidewalks - Grinde TBS



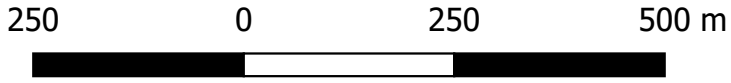
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Tactical Level Asset Management Study (Phase 2)
Water Distribution, Wastewater Collection, and Roadways

2017-04-06

Map 2 - Sidewalk Treatments





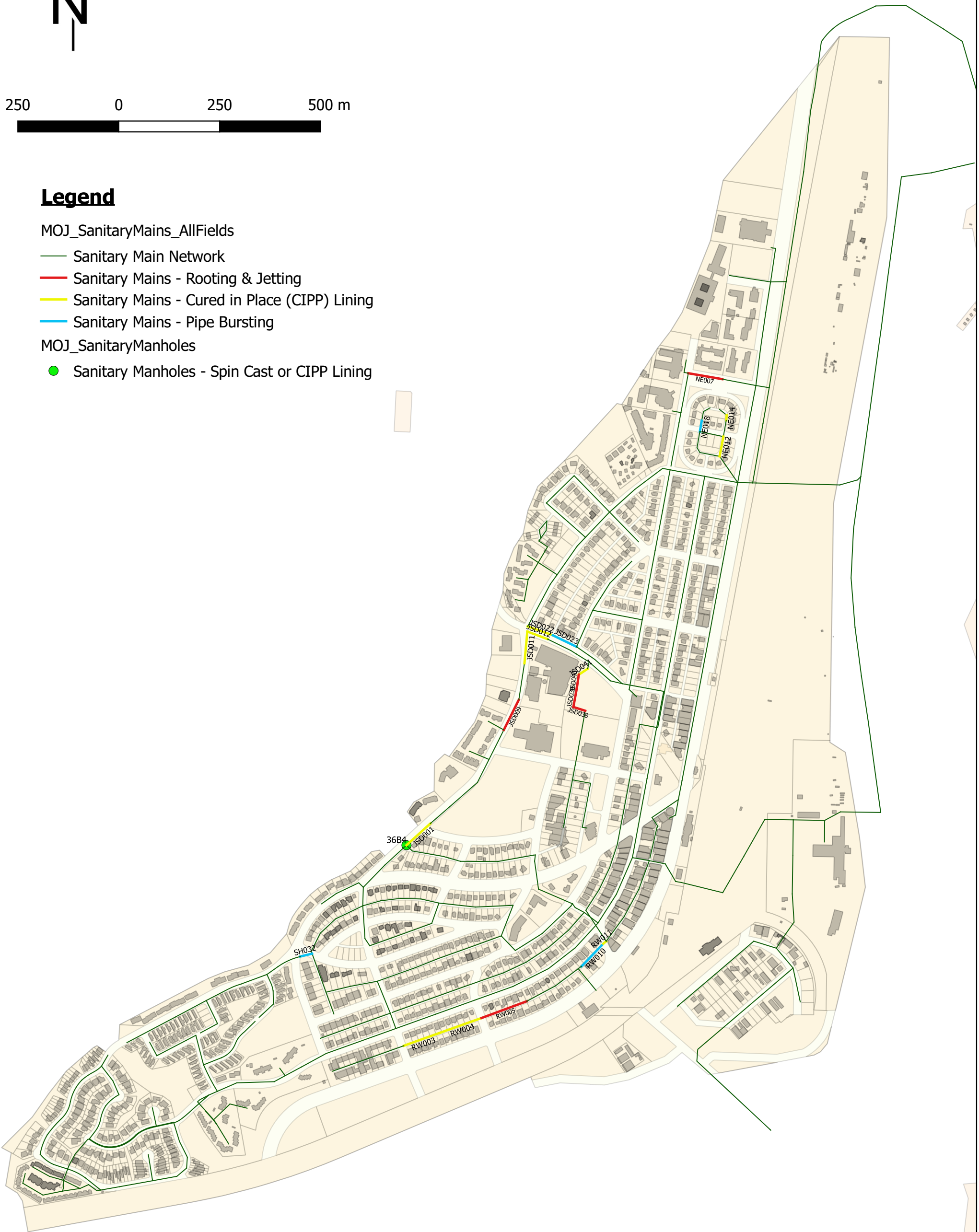
Legend

MOJ_SanitaryMains_AllFields

- Sanitary Main Network
- Sanitary Mains - Rooting & Jetting
- Sanitary Mains - Cured in Place (CIPP) Lining
- Sanitary Mains - Pipe Bursting

MOJ_SanitaryManholes

- Sanitary Manholes - Spin Cast or CIPP Lining



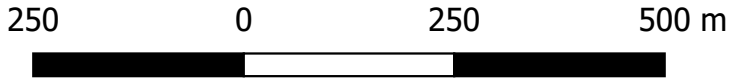
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2017-04-06

Map 3 - Wastewater System Treatments



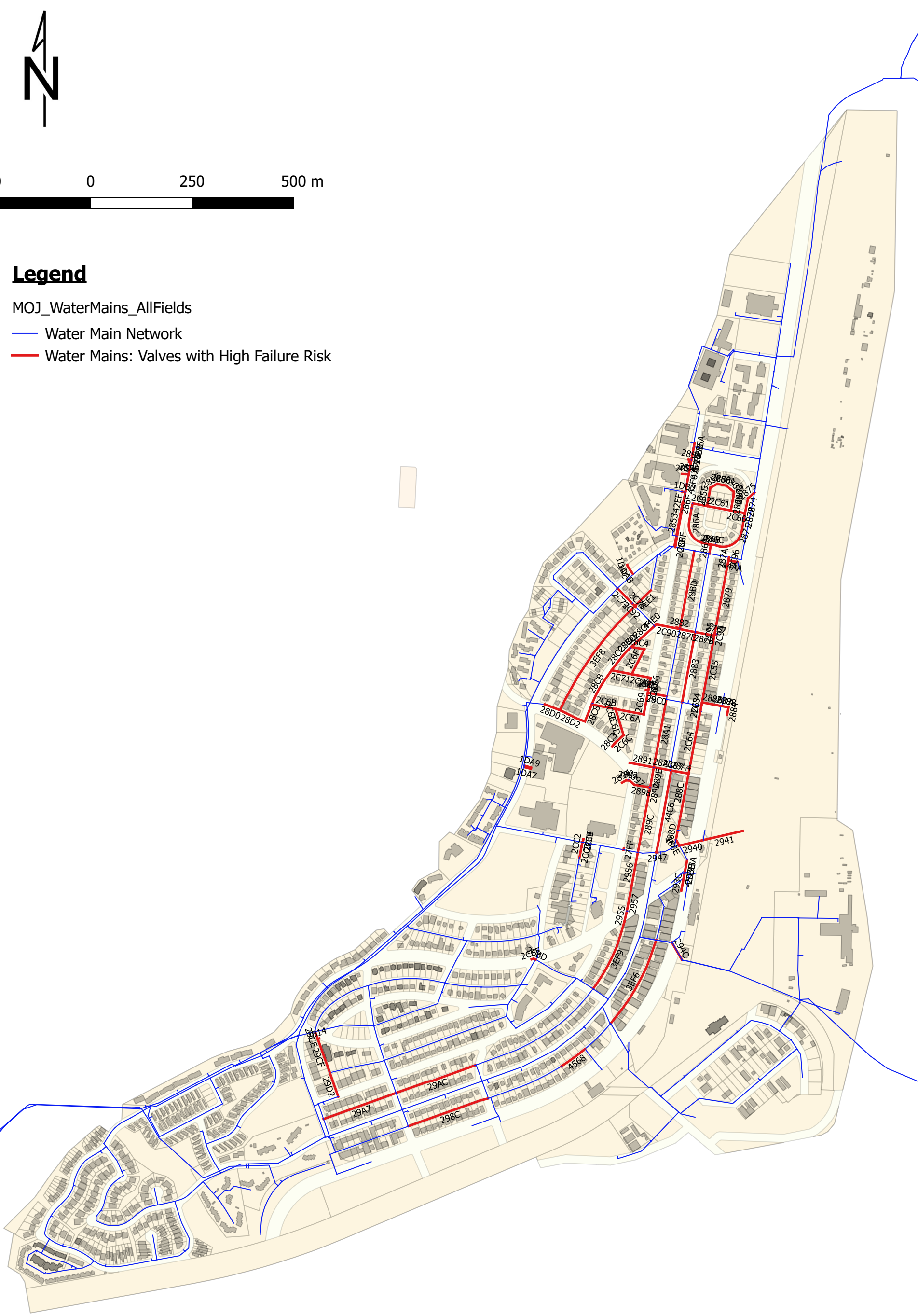


Legend

MOJ_WaterMains_AllFields

— Water Main Network

— Water Mains: Valves with High Failure Risk



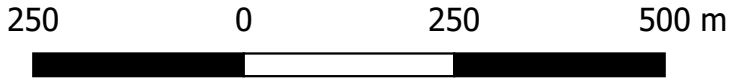
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2017-04-06

Map 4 - Water Distribution System: Valve Treatments





Legend

MOJ_WaterMains_AllFields

— Water Main Network

— Water Mains: Valves with High Failure Risk

See Map 4a for detail

See Map 4b for detail

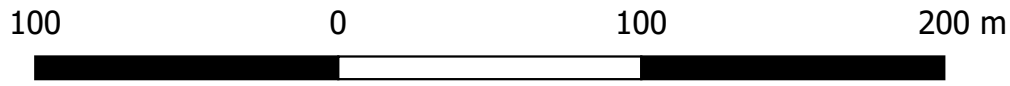
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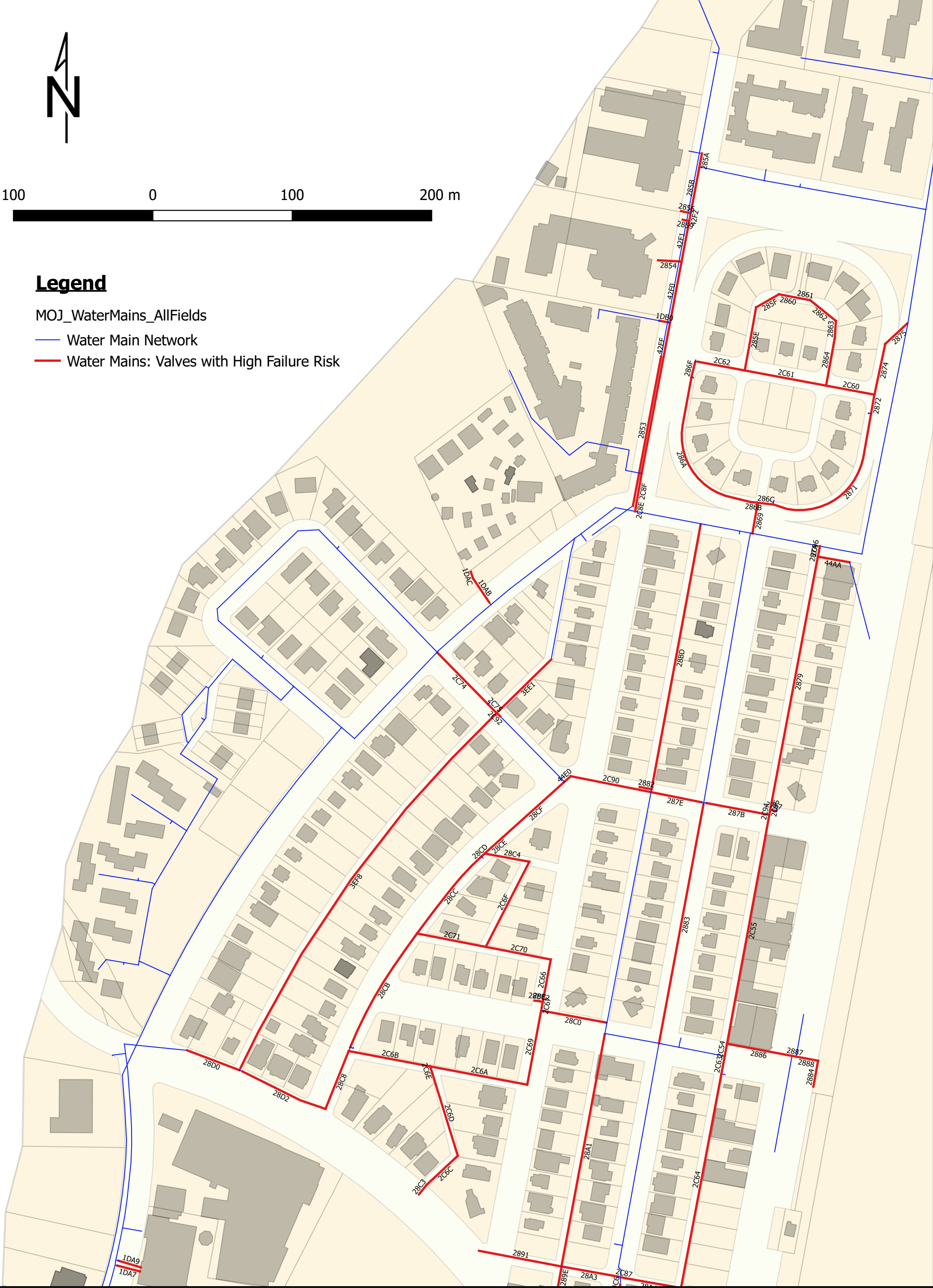
Map 4 - Water Distribution System: Valve Treatments





Legend

- MOJ_WaterMains_AllFields
- Water Main Network
- Water Mains: Valves with High Failure Risk



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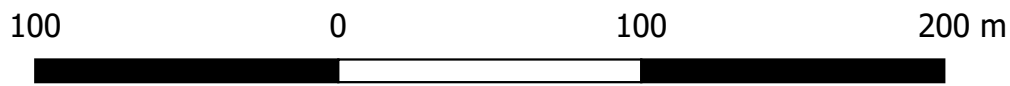
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2017-04-06

Map 4a - Water Distribution System: Valve Treatments



Rev.2 - 2017-04-04

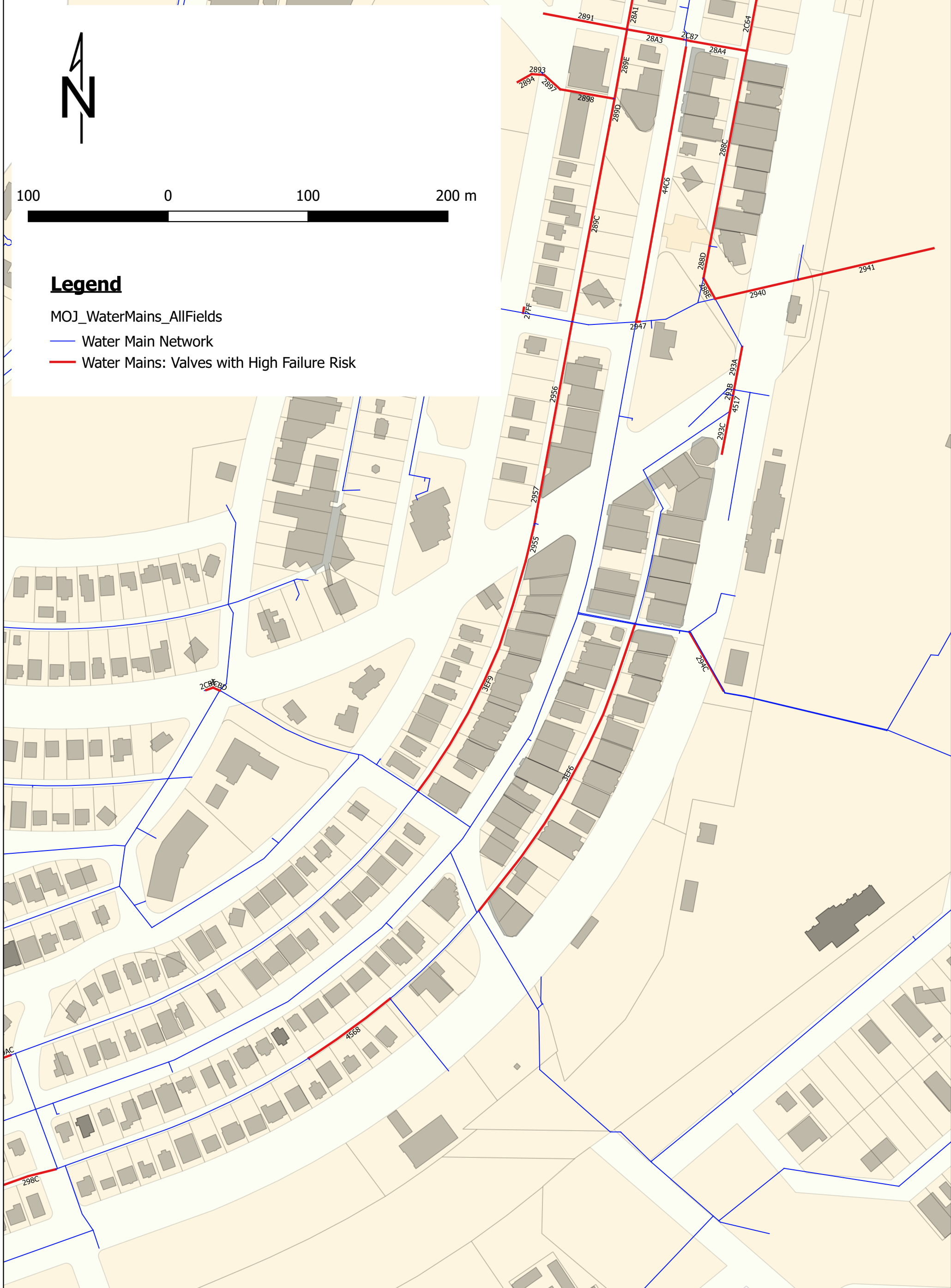


Legend

MOJ_WaterMains_AllFields

— Water Main Network

— Water Mains: Valves with High Failure Risk



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Map 4b - Water Distribution System: Valve Treatments

